

EXISTING	LEGEND	PROPOSED
---	STORM SEWER	---
---	SANITARY SEWER	---
---	WATERMAIN	---
---	GAS MAIN	---
---	ELEC. TELE. CABLE	---
---	STORM MANHOLE	○
---	CATCH BASIN	□
---	INLET	○
---	REARYARD CATCH BASIN	□
---	END SECTION	---
---	SANITARY MANHOLE	○
---	GATE VALVE AND WELL	○
---	HYDRANT	○
---	CONTOURS	---
---	MATCH ELEVATION	---
---	TOP OF CURB	(M)
---	TOP OF SIDEWALK	T/C
---	TOP OF PAVEMENT	T/S
---	TOP OF CONCRETE	T/P
---	STANDARD CURB	---
---	REVERSE CURB	---
---	DOWNSPOUT	---
---	ROOF DRAIN	---
---	D.S.	---
---	R.D.	---

(SEE SHEET 5)

DATE	ISSUE
3-19-2021	REVISED PER ARCHITECT
5-12-2021	REVISED PER CITY (4-15-2021)
6-30-2021	REVISED PER CITY (6-29-2021)
9-14-2021	NO CHANGES

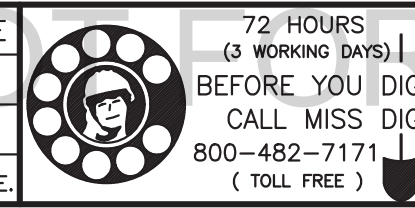
Developer/Contractor
 OYK ENGINEERING & CONSTRUCTION
 30700 TELEGRAPH ROAD, SUITE 2665
 BINGHAM FARMS, MICHIGAN 48025
 (248) 656-7695 FRED HADDI

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 PHONE (248) 625-5251 www.kiefteng.com FAX (248) 625-7110

DATE	CKD. BY	DATE
3-17-2021		



Detailed Site Plan - West
 "BEBB OAK MEADOWS" MIXED USE DEVELOPMENT
 PART OF THE SE 1/4 OF SECTION 27, T3N, R11E,
 CITY OF ROCHESTER HILLS, OAKLAND COUNTY, MICHIGAN

SCALE	SHEET
1" = 20'	5 OF 9

KE 2021.053

Not to be Used as Construction Drawings

City File #21-008, Section 27

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outccad.your.job.name

"C" Calculations:

TOTAL SITE AREA = 4.71 ACRES
 GREENBELTS = 1.09 ACRES @ C=0.25
 PAVEMENT = 2.92 ACRES @ C=0.95
 ROOFS = 0.70 ACRES @ C=0.95

$$C = \left(\frac{0.70 \text{ Ac} \times 0.95}{4.71 \text{ Acres}} \right) + \left(\frac{2.92 \text{ Ac} \times 0.95}{4.71 \text{ Acres}} \right) + \left(\frac{1.09 \text{ Ac} \times 0.25}{4.71 \text{ Acres}} \right) = 0.788 = 0.79$$

25 YEAR DETENTION BASIN CALCULATIONS (i.e. WITH OUTLET)

ONSITE CONTRIBUTING AREA = 4.71 ACRES
 OFFSITE CONTRIBUTING AREA = 0.00
 TOTAL CONTRIBUTING AREA = 4.71 ACRES

RUNOFF COEFFICIENT (C) = 0.79
 $Q_A = (0.20)(4.71 \text{ ACRES}) = 0.942 \text{ C.F.S.}$
 $Q_O = \frac{Q_A}{(A)(C)} = \frac{0.942}{(4.71)(0.79)} = 0.253$

$T = -25 + \sqrt{\frac{8.062 \cdot 50}{0.60}} = 153.46 \text{ MINUTES}$
 $V_S = \frac{12,900 \cdot T}{T + 25} - 40 \cdot Q_O \cdot T = 9,539 \text{ C.F./ACRE}$

$V_T \text{ REQ'D} = V_S \cdot (C)(A) = (9,539)(0.79)(4.71) = 35,494 \text{ C.F. REQ'D.}$

VOLUME PROVIDED: USING 42" DIAMETER PIPE
 (3,784 LF) x (9.621 CF/LF) = 36,405 CF PROVIDED

ORIFICE FORMULA

$Q_A = 0.62 (A_O)(2gh)^{1/2}$
 $A_O = \text{AREA OF ORIFICE PIPE}$
 $g = 32.2 \text{ FT./SEC.}^2$
 $h = \text{DEPTH OF BASIN ABOVE CENTERLINE OUTLET PIPE} = [760.80 - (759.92 + \frac{5 \cdot 1}{2}) = 0.65']$
 $A_O = 0.62 (2gh)^{1/2} = \frac{0.942 - 0.292}{0.62 [(2)(32.2)(0.65)]^{1/2}} = 0.1620 \text{ SF}$

DIAMETER ORIFICE (D_O): $D_O = \sqrt{\frac{4(A_O)}{\pi}} = \sqrt{\frac{4(0.1620)}{\pi}} = 0.454 \text{ FT.} = 5.45 \text{ INCH}$
 HOWEVER, USE 5 1/2" RESTRICTION

THUS, $Q_A = (0.62)(0.1620)(\sqrt{(2)(32.2)(0.65)}) = 0.65$

UNDERGROUND DETENTION BASIN (42" DIAMETER PIPES):

EXISTING WATER ELEVATION = NONE
 DESIGN HIGH WATER ELEVATION = 760.80
 1.00' FREEBOARD ELEVATION = 761.80
 STORAGE REQUIRED = 35,494 CF
 STORAGE PROVIDED = 36,405 CF (BETWEEN 757.30 UPTO 760.80)
 TIME TO DRAIN = 35,494 CF / 0.942 CFS = 37,679 SECONDS / 3600 SEC/HR = 10.47 HOURS

BANK FULL VOLUME CALCULATION:

VOLUME REQUIRED = 6,788(A)(C)
 = (6,788)(4.71 Ac)(0.79) = 25,258 CF

REQUIRED AREA PER LF = 25,258 CF / 3,784 LF (OF 42" PIPE) = 6.68 SF
 (3.5') 42" DETENTION PIPE INVERT = 757.30
 REQUIRED WATER DEPTH = 75% (3.5') = 2.62' + 757.30 = 759.92

BANK FULL ORIFICE FORMULA

$Q_A = 0.62 (A_O)(2gh)^{1/2}$
 $A_O = \text{AREA OF ORIFICE PIPE}$
 $g = 32.2 \text{ FT./SEC.}^2$
 $h = \text{DEPTH OF BASIN ABOVE CENTERLINE OUTLET PIPE} = [759.92 - (757.30 + \frac{2.75}{2}) = 2.39']$
 $A_O = 0.62 (2gh)^{1/2} = \frac{0.292}{0.62 [(2)(32.2)(2.39)]^{1/2}} = 0.0379 \text{ S.F.}$

DIAMETER ORIFICE (D_O): $D_O = \sqrt{\frac{4(A_O)}{\pi}} = \sqrt{\frac{4(0.0379)}{\pi}} = 0.2198 \text{ FT.} = 2.64 \text{ INCH}$
 USE 2.75" MINIMUM RESTRICTION

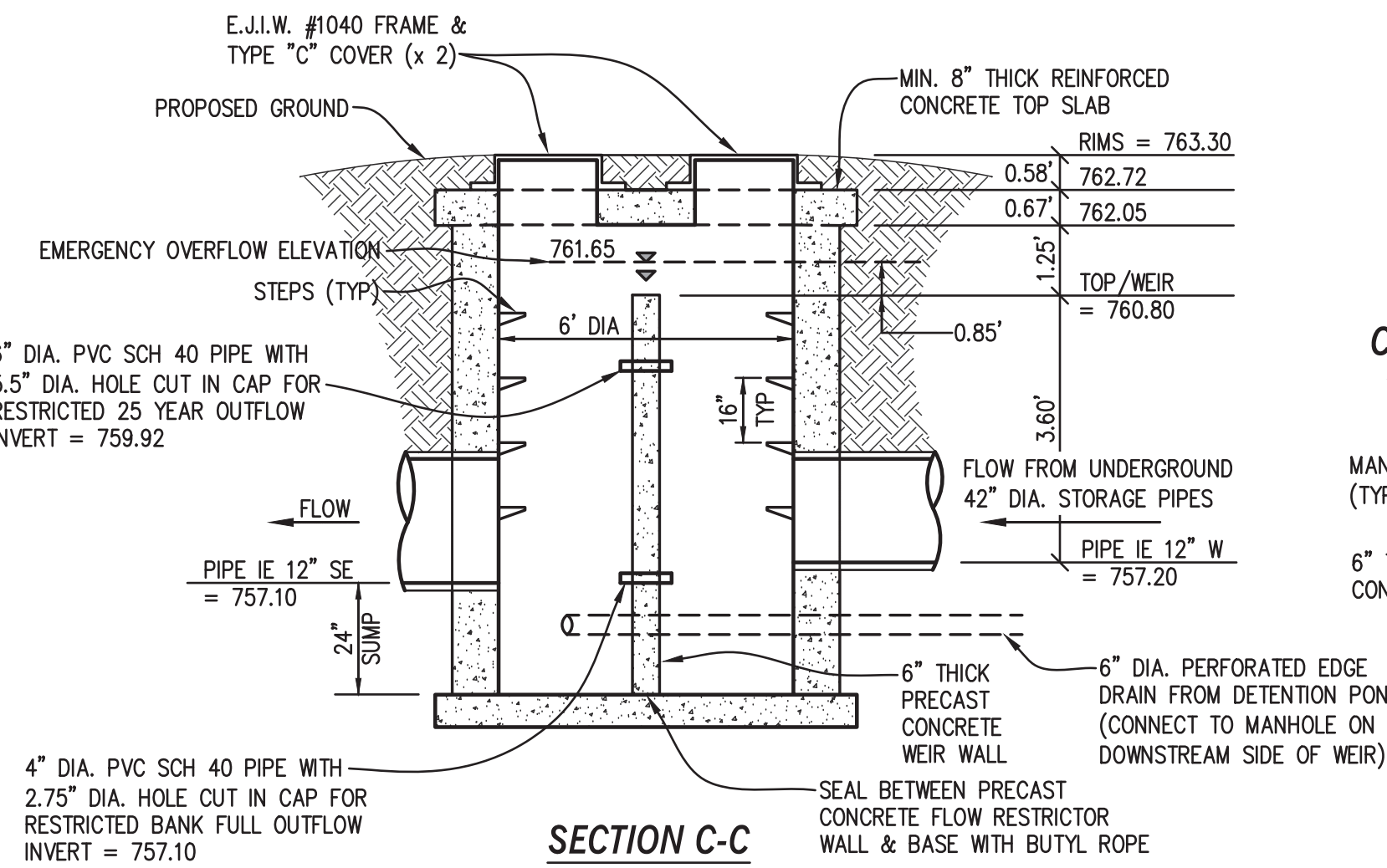
THUS, $Q_A = (0.62)(0.0379)(\sqrt{(2)(32.2)(2.39)}) = 0.292$

RESTRICTION CALCULATION:

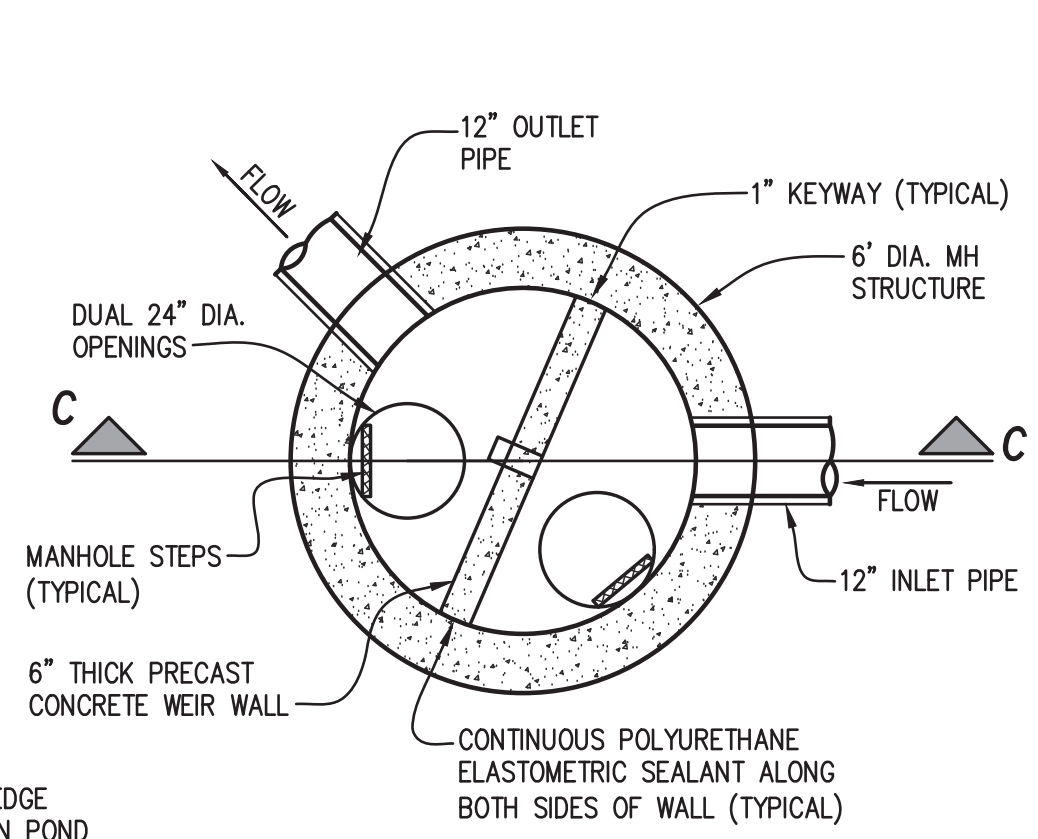
OC Restrictor Sizing:
 Storage Elevation = 760.80
 Outlet Elevation = 757.30
 Hmax = 3.50 ft.
Bank Full Outlet Restriction:
 Bank Full Volume = 6,788 A*C = 25,258 cf
 Required Area per LF = 6.68 sf
 Detention Pipe Invert = 757.30
 Water Depth in Pipe = 2.62 ft
 Zbf = 759.92
 Release Between 24 and 48 Hours:
 Qavg (24 hours) = 0.292 cfs
 Havg (0.667*Zbf-Zout) = 1.75 ft
 $A = (Q_{avg} / ((0.62 * (32.2 * H_{avg})^{0.5}))) = 0.0379 \text{ sf}$
 BF Hole Dia. Provided = 2.75 in
 BF Hole Area Provided = 0.0379 sf
 Qact bf = 0.292 cfs
 Actual Holding Time = (Vbf/Qact*3600) = 24.03 hrs
25 Year Outlet Restriction:
 Qallow = 0.942 cfs
 Qbf = 0.292 cfs
 Qadjusted = (Qallow/Qbf) = 0.65
 $A = (Q_{adjusted} / ((0.62 * (32.2 * H_{max})^{0.5}))) = 0.1620 \text{ sf}$
 25 Year Hole Dia. Provided = 5.50 in
 25 Year Hole Area Provided = 0.1620 sf
 Qactual (total) = 0.942 cfs <= 0.942

10 YEAR SPILLWAY DESIGN (WITHIN MH-1)

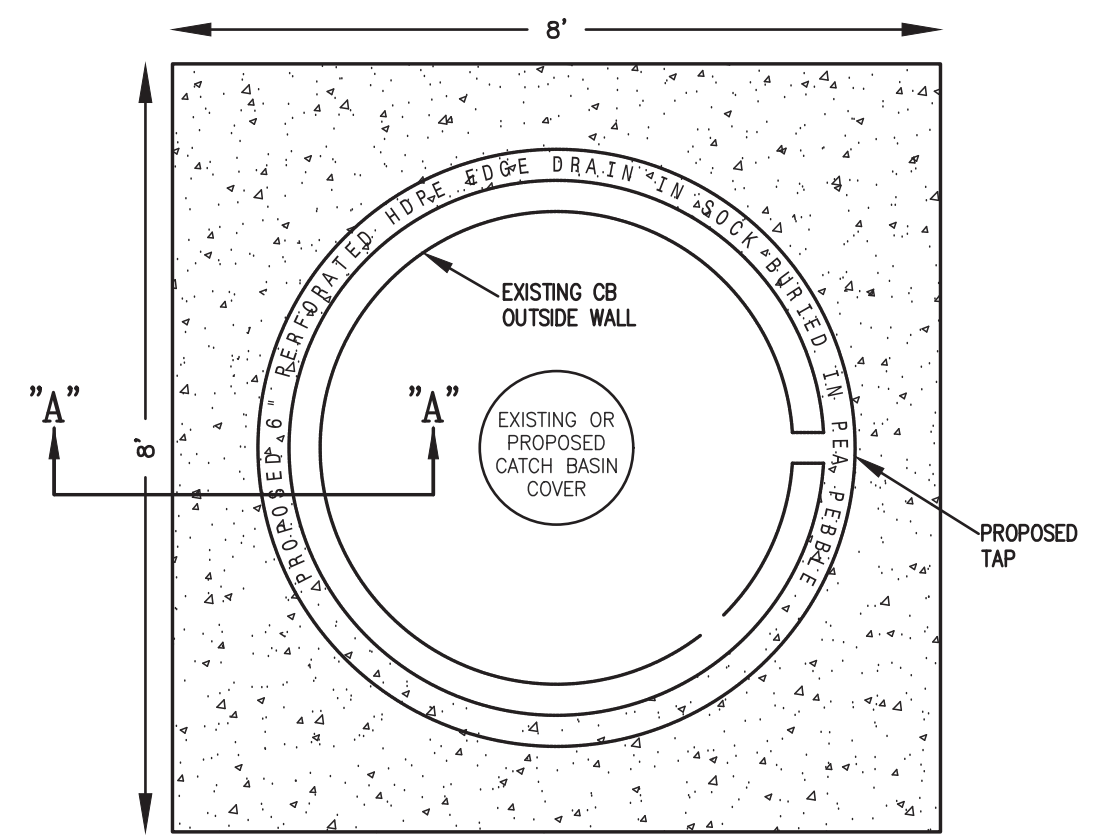
1:10 YR = 175 : T = 17.00; I = 175 / 17.00 = 4.17
 $Q \text{ REQUIRED} = ACI = (4.71 \text{ Ac.})(0.79)(4.17) = 15.52 \text{ C.F.S. REQUIRED}$
 $Q \text{ PROVIDED} = 3.367 \text{ LH } 3/2; \text{ TRY } H = 0.50'$
 $Q = (3.367)(6')(0.85)^{3/2} = 15.83 \text{ CFS PROVIDED, OK!}$



6' DIA. OUTLET CONTROL STRUCTURE MH-1 DETAIL

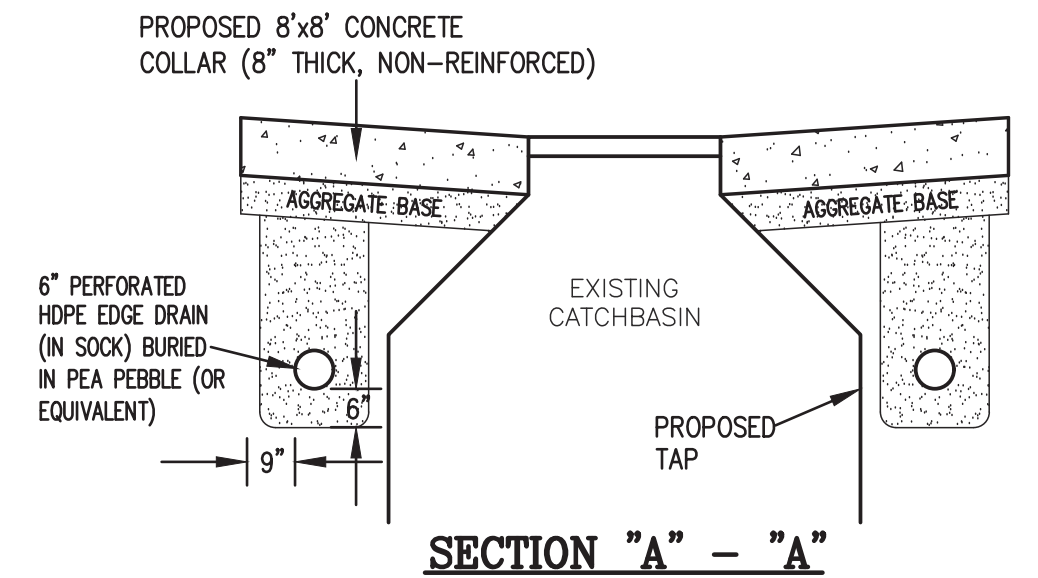


PLAN VIEW

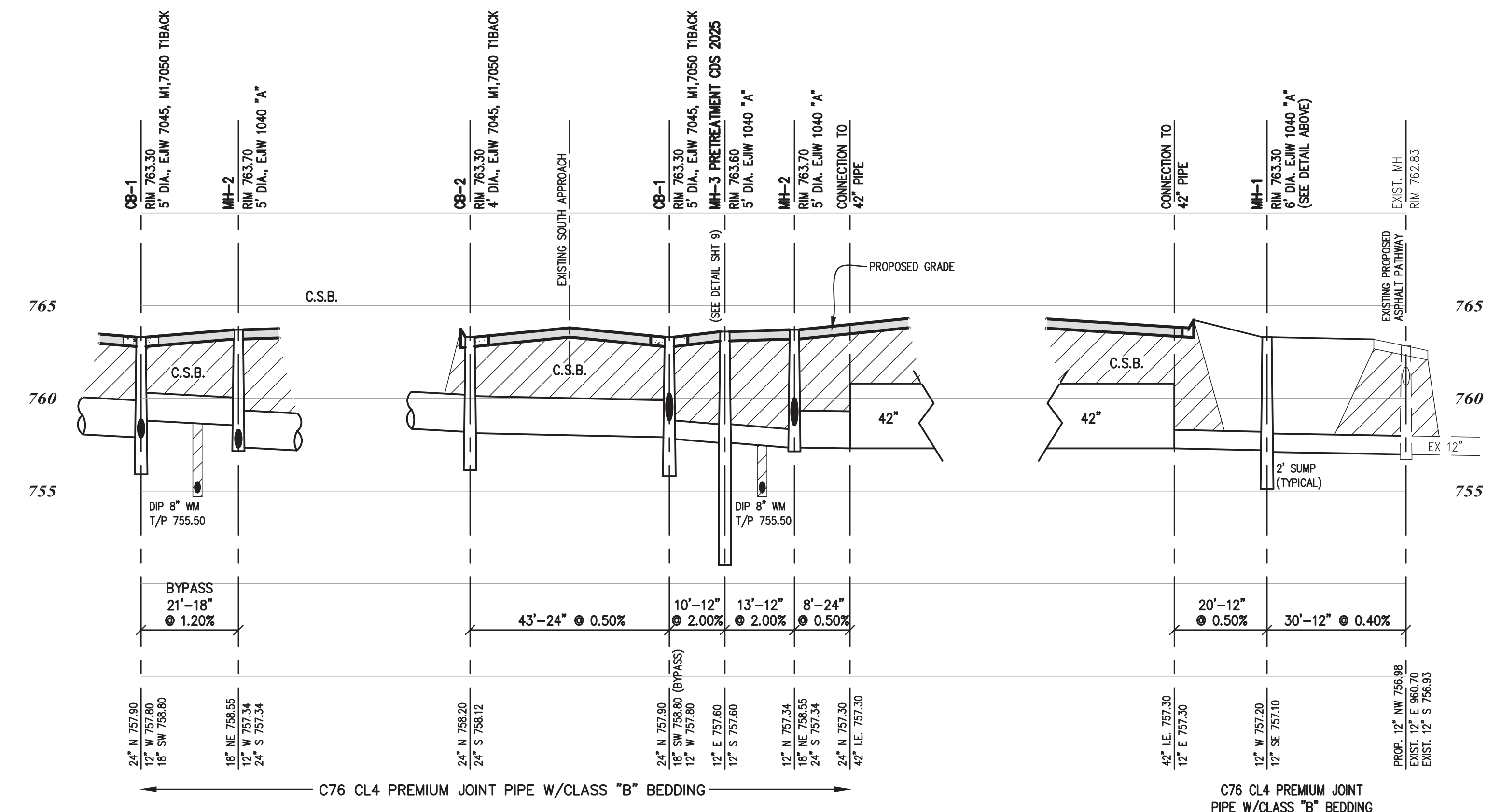


8" CONCRETE COLLAR & 6" EDGE DRAIN PLAN DETAIL & SECTION

SCALE: 1" = 2'



SECTION "A" - "A"



SCALE: 1" = 20' HORIZ.
 1" = 5' VERT.

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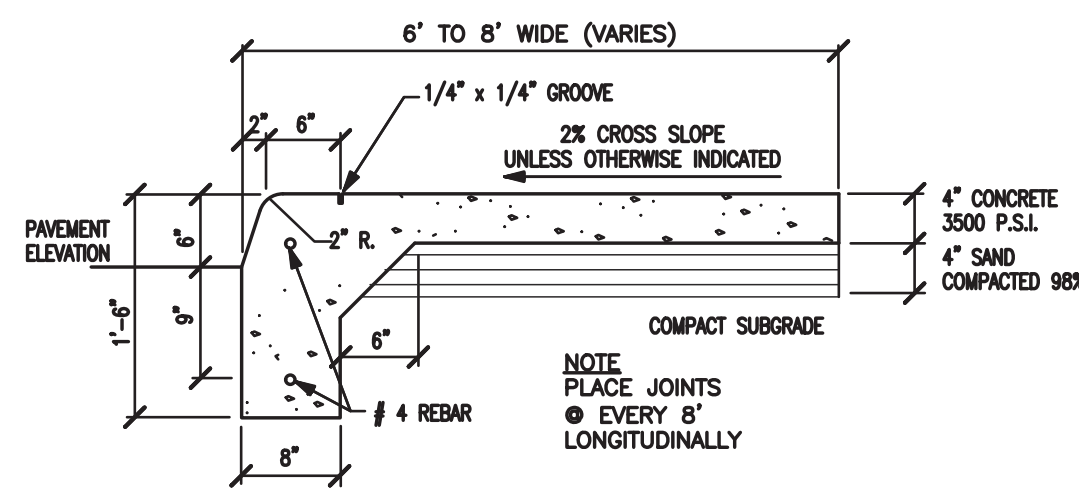


Not to be Used as Construction Drawings
Storm Sewer, Detention Calculations & Details
 "BEBB OAK MEADOWS" MIXED USE DEVELOPMENT
 PART OF THE SE 1/4 OF SECTION 27, T3N, R11E,
 CITY OF ROCHESTER HILLS, OAKLAND COUNTY, MICHIGAN

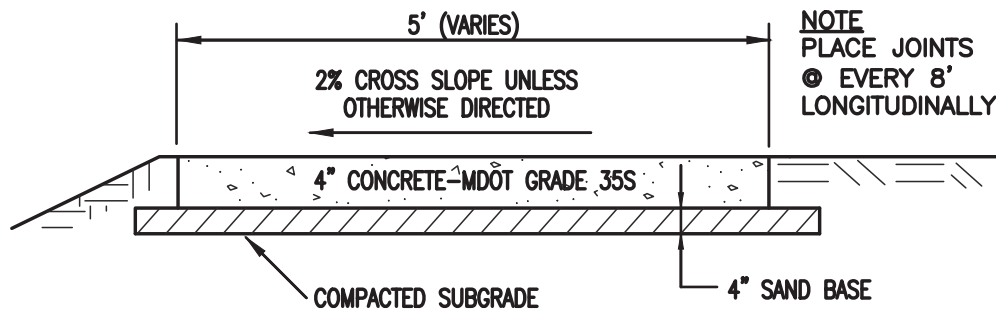
SCALE	N/A
SHEET	7 OF 9

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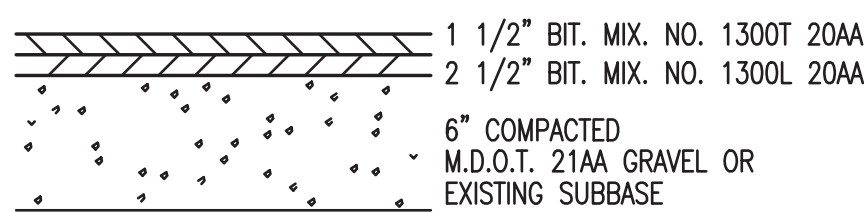
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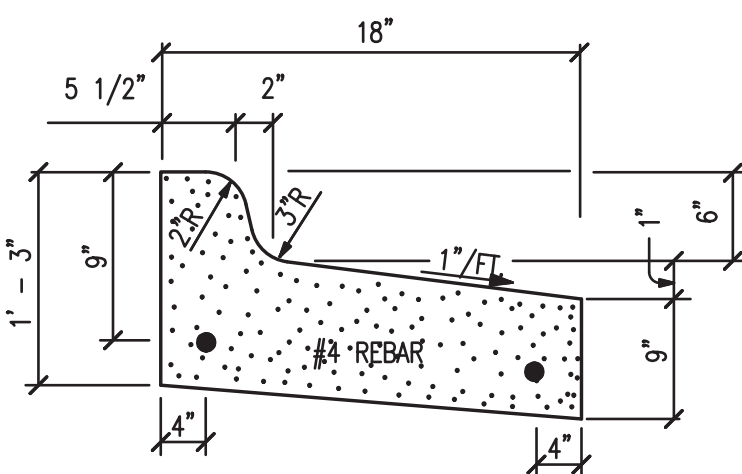
INTEGRAL CURB & WALK
NO SCALE



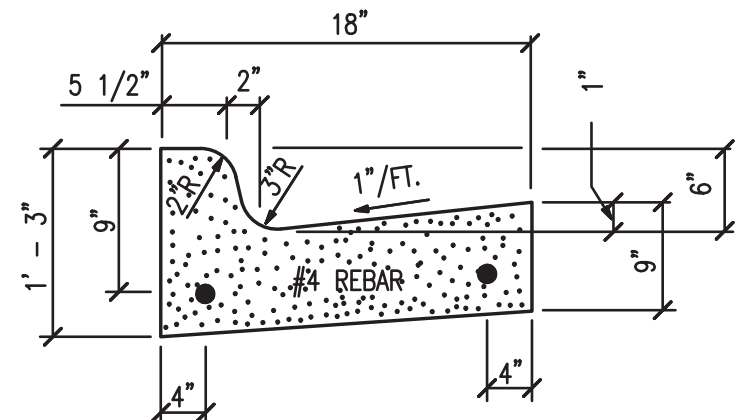
CONCRETE SIDEWALK (ON-SITE)
NO SCALE



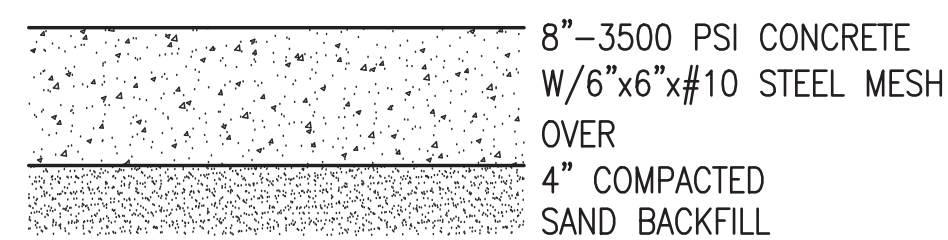
PAVEMENT SECTION-ONSITE
NO SCALE



STRAIGHT FACED CURB AND GUTTER (REVERSE)
NO SCALE



STRAIGHT FACED CURB AND GUTTER
NO SCALE



8\"/>

ON-SITE SANITARY SEWER DESIGN

APARTMENT BUILDING: 93 UNITS
UNIT FACTOR = (0.60/UNIT) x (93 UNITS) = 55.8

GENERAL RETAIL BUILDING: 9,660 SF
UNIT FACTOR = (0.04/1000 SF) x (9,660 SF) = 0.39

DRIVE-THRU RESTAURANT: 3,782 SF
UNIT FACTOR = 1.00

TOTAL UNIT FACTOR = 55.8+0.39+1.00 = 57.19 = 58

THUS, (58 UNIT FACTOR)(2.44 PERSONS/UNIT FACTOR) = 142 PERSONS

SANITARY DESIGN

"AVERAGE FLOW"

$$142 \text{ PERSONS} \times 100 \text{ GPCD} = 14,200 \text{ GPD} = 0.0142 \text{ MGD}$$

$$Q = 14,200 \text{ GPD} \times 1.55 \text{ (conversion)} = 0.0220 \text{ CFS}$$

"PEAK FLOW"

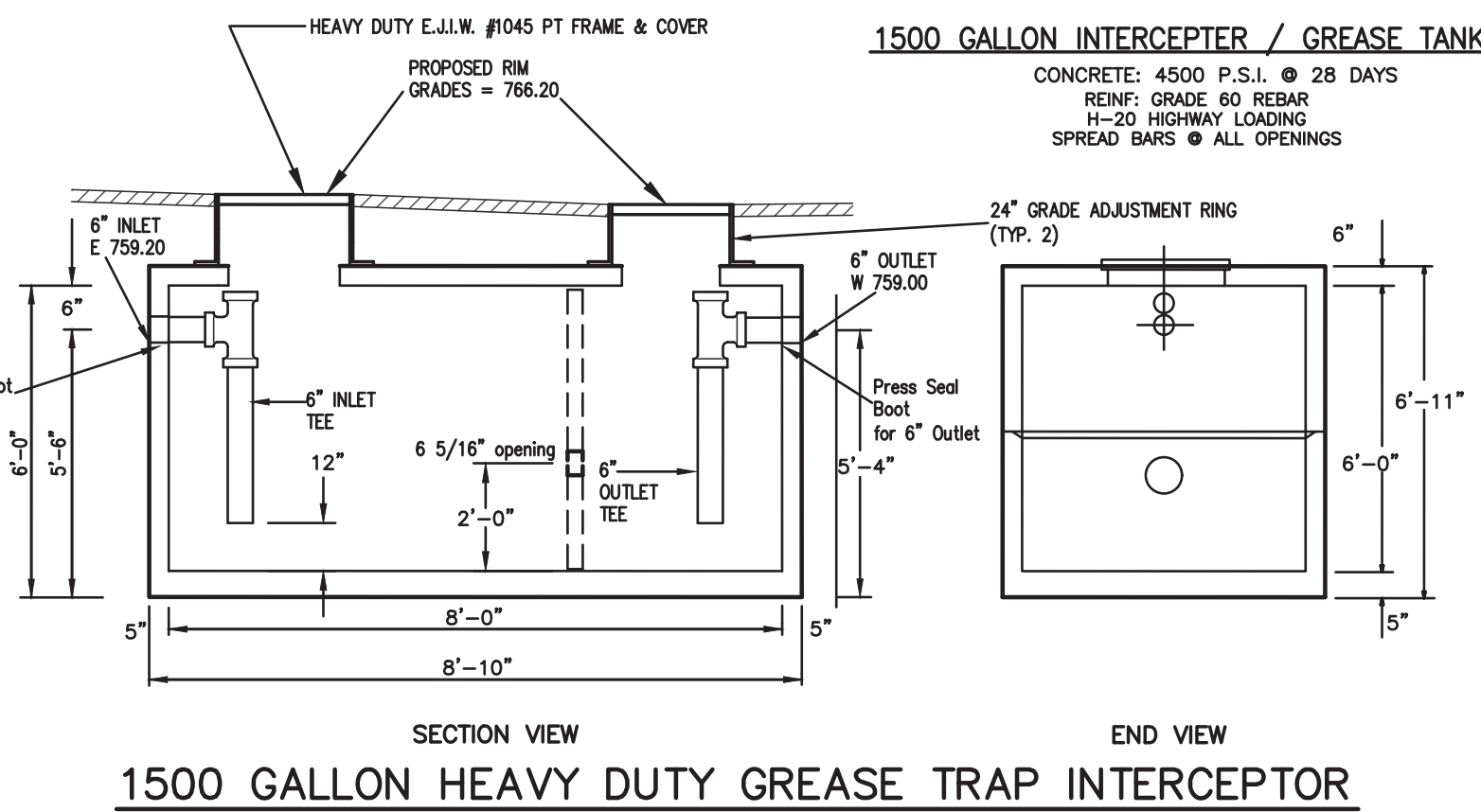
$$142 \text{ PERSONS}/1000 = 0.142$$

$$Q = 100 \times (18 + \sqrt{P}) / (4 + \sqrt{P}) = 419.87 \text{ GPCD}$$

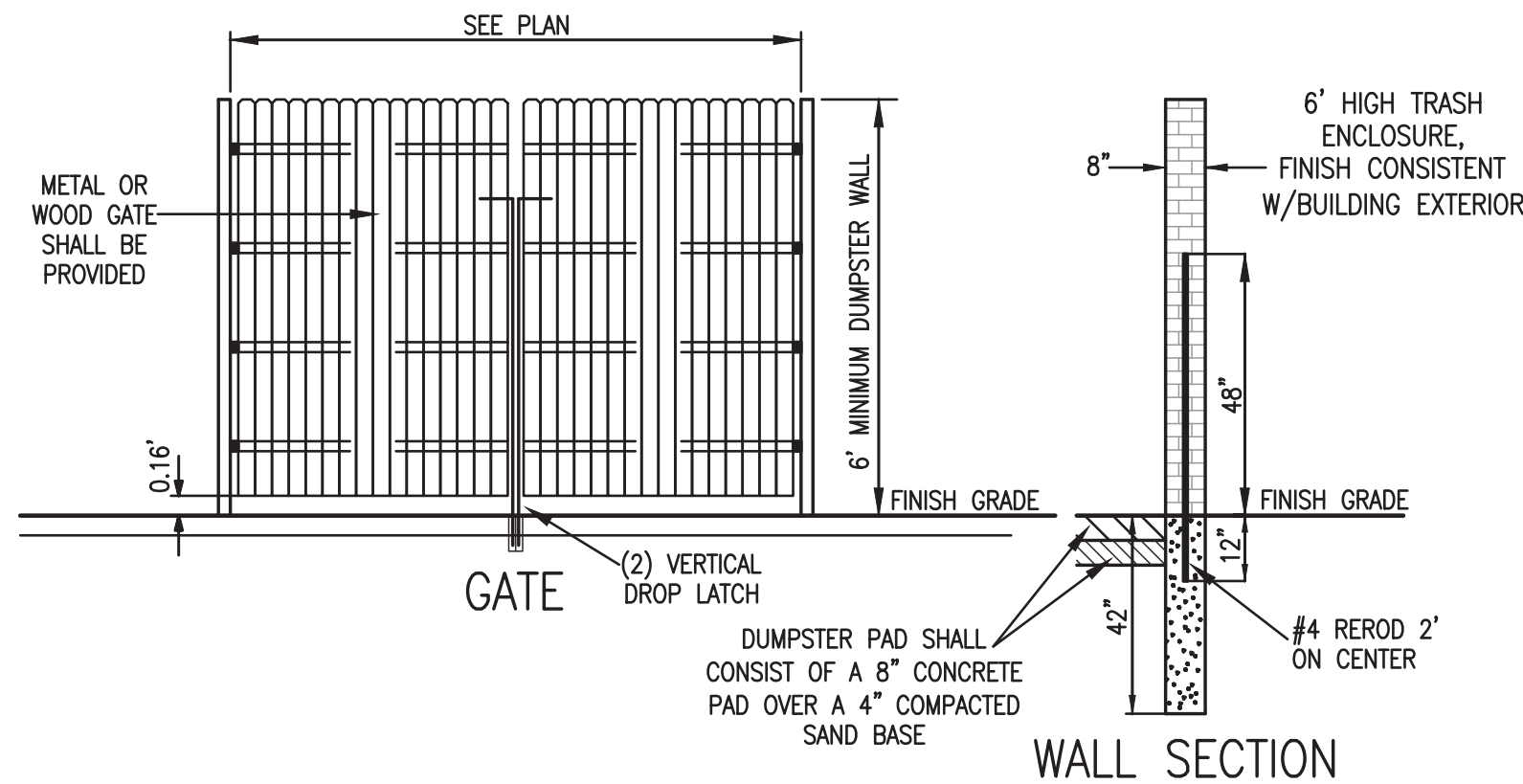
$$142 \text{ PERSONS} \times 419.87 \text{ GPCD} = 59,621.54 \text{ GPD} = 0.059621 \text{ MGD}$$

$$Q = 0.059621 \text{ MGD} \times 1.55 \text{ (conversion)} = 0.0924 \text{ CFS}$$

NOTE: AN 8" TRUSS PIPE @ 0.40% (MINIMUM) CAN CARRY 0.765 CFS @ 2.19 FPS
A 10" TRUSS PIPE @ 0.30% (MINIMUM) CAN CARRY 1.19 CFS @ 2.19 FPS



1500 GALLON HEAVY DUTY GREASE TRAP INTERCEPTOR



Dumpster Detail
NOT TO SCALE

Project: Mixed Use Development
Location: Rochester Hills, MI
Prepared For: Kieft Engineering, Inc.



Purpose: To calculate the first flush runoff flow rate (WQV) for a given site area. In this situation the WQV to be analyzed is the runoff produced by the first 0.5" of rainfall.

Reference: United States Department of Agriculture Natural Resources Conservation Service TR-55 Manual

Structure Name	A (acres)	A (miles ²)	Runoff Coefficient	Percent Imp. (%)	L (min)	L (hr)
WQU	4.71	0.00736	0.79	81.67	15.0	0.250

* Assumes runoff coefficient of 0.3 for pervious areas and 0.9 for impervious areas.

Procedure: The Water Quality Flow (WQF) is calculated using the Water Quality Volume (WQV). This WQV, converted to watershed inches, is substituted for the runoff depth (Q) in the Natural Resources Conservation Service (formerly Soil Conservation Service), TR-55 Gr

1. Compute WQV in watershed inches using the following equation:

$$WQV = P \times R$$

where: WQV = water quality volume (watershed inches)
P = design precipitation (inches)
R = volumetric runoff coefficient = (0.05 + 0.009I)
I = percent impervious cover

Structure Name	Percent Imp. (%)	R (in)	P (in)	WQV (in)	WQV (CF)
WQU	81.67	0.785	0.5	0.393	6,710.69

2. Compute the NRCS Runoff Curve Number (CN) using the following equation, or graphically using Figure 2-1 from TR-55 (USDA, 1986):

$$CN = 1000 / (10 + 5P + 10Q - 10(Q + 2.25P)^{0.7})$$

where: CN = Runoff Curve Number
P = design precipitation (inches)
Q = runoff depth (watershed inches)

Structure Name	Q (in)	CN
WQU	0.393	88.96

3. Using computed CN, read initial abstraction (I_a) from Table 4-1 in Chapter 4 of TR-55; compute I_aP, interpolating when appropriate.

Structure Name	I _a (in)	I _a P
WQU	0.041	0.062

4. Compute the time of concentration (L_c) in hours and the drainage area in square miles. A minimum L_c of 0.167 hours (10 minutes) should be used.

Structure Name	L _c (hr)	A (miles ²)
WQU	0.250	0.00736

5. Read the unit peak discharge (q_u) from Exhibit 4-11 in Chapter 4 of TR-55 for appropriate L_c for type II rainfall distribution.

Structure Name	L _c (hr)	I _a P	q _u (csm/min)
WQU	0.250	0.062	731

6. Substituting WQV (watershed inches) for runoff depth (Q), compute the water quality flow (WQF) from the following equation:

$$WQF = (q_u I_a P) / Q$$

where: WQF = water quality flow (cfs)
q_u = unit peak discharge (csm²/inch)
A = drainage area (mi²)
Q = runoff depth (watershed inches)

Structure Name	q _u (csm/min)	A (miles ²)	Q (in)	WQF (cfs)
WQU	731	0.00736	0.393	2.11

Estimated Net Annual Solids Load Reduction Based on the Rational Rainfall Method

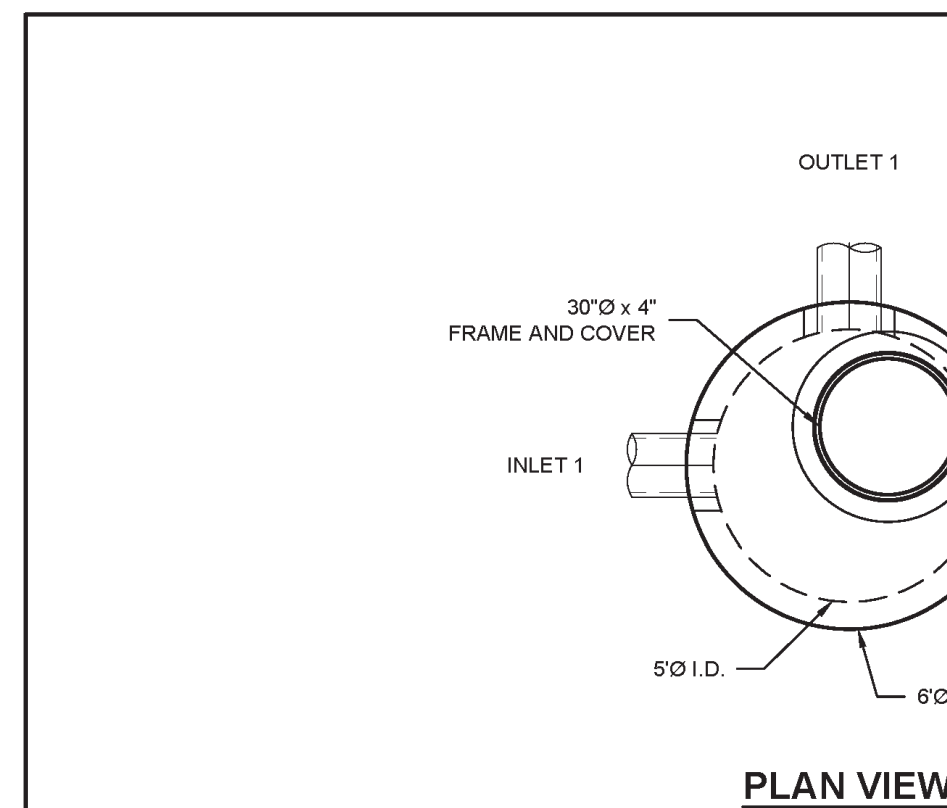
Mixed Use Development
Rochester Hills, MI
Water Quality Unit

AREA (acres): 4.71
WEIGHTED C: 0.79
Tc (minutes): 15

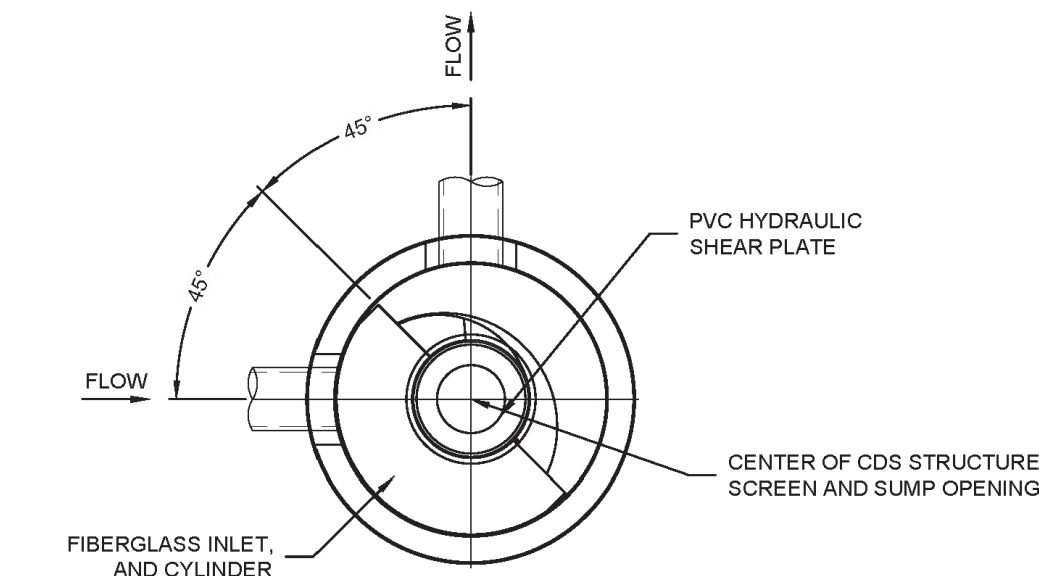
CDS MODEL: 2025
PARTICLE SIZE (µm): 110

Rainfall Intensity (in/hr)	Percent Rainfall Volume ¹	Cumulative Rainfall Volume	Total Flowrate (cfs)	Removal Efficiency (%)	Incremental Removal (%)
0.02	13.13%	13.13%	0.07	100.00	13.13
0.04	11.36%	24.50%	0.15	99.17	11.27
0.06	10.09%	34.60%	0.22	98.07	9.88
0.08	7.49%	42.10%	0.30	96.96	7.28
0.10	7.01%	49.10%	0.37	95.86	6.72
0.12	5.37%	54.48%	0.45	94.75	5.99
0.14	4.73%	59.20%	0.52	93.65	4.43
0.16	4.13%	63.33%	0.60	92.54	3.82
0.18	3.53%	66.80%	0.67	91.44	3.23
0.20	2.99%	69.80%	0.74	90.33	2.70
0.25	5.50%	75.30%	0.93	87.57	4.62
0.30	4.47%	79.80%	1.12	84.81	3.79
0.35	3.85%	83.65%	1.30	82.04	3.16
0.40	2.18%	85.85%	1.49	79.3	1.7
0.45	2.09%	87.95%	1.67	74.2	1.6
0.50	1.31%	89.26%	1.86	66.8	0.9
0.75	5.07%	94.33%	2.79	44.5	2.3
1.00	2.56%	96.89%	3.72	33.4	0.9
1.50	2.50%	99.4%	5.58	22.3	0.6
2.00	0.81%	99.9%	7.44	13.1	0.0
2.54	0.15%	100.0%	9.45	13.1	0.0
				Predicted % Annual Rainfall Treated =	93.8%
				Predicted Net Annual Load Removal Efficiency =	87.2%

1 - Based on Rainfall Data from DETROIT METRO AP Station
2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.



PLAN VIEW



SECTION A-A

MATERIAL LIST (PROVIDED BY CONTECH)

COUNT	DESCRIPTION	INSTALLED BY
1	FIBERGLASS INLET AND CYLINDER	CONTECH
1	2400 micron, 2' O.D. x 2.58' SEP. SCREEN	CONTECH
1	3/16 INCH PVC HYDRAULIC SHEAR PLATE *	CONTECH
1	SEALANT FOR JOINTS	CONTRACTOR
1	30"Ø x 4' FRAME & COVER, E.J.#1600484, OR EQUIV.	CONTRACTOR

* SEE HYDRAULIC SHEAR PLATE DETAIL

GENERAL NOTES

- CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- FOR FABRICATION DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE. www.ContechES.com
- CDS WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT.
- STRUCTURE SHALL MEET AASHTO HS-20 LOAD RATING, ASSUMING EARTH COVER OF 0'-2" AND GROUNDWATER ELEVATION AT OR BELOW THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M308 AND BE CAST WITH THE CONTECH LOGO.
- IF REQUIRED, PVC HYDRAULIC SHEAR PLATE IS PLACED ON SHELF AT BOTTOM OF SCREEN CYLINDER. REMOVE AND REPLACE AS NECESSARY DURING MAINTENANCE CLEANING.
- CDS STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C-478 AND AASHTO LOAD FACTOR DESIGN METHOD.

INSTALLATION NOTES

- ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE CDS MAINHOLE STRUCTURE.
- CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS AND ASSEMBLE STRUCTURE.
- CONTRACTOR TO PROVIDE, INSTALL, AND GROUT INLET AND OUTLET PIPE(S). MATCH PIPE INVERTS WITH ELEVATIONS SHOWN. ALL PIPE CENTERLINES TO MATCH PIPE OPENING CENTERLINES.
- CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.

STRUCTURE WEIGHT
APPROXIMATE HEAVIEST PICK = 9500 LBS.
STRUCTURE IS DELIVERED IN 3 PIECES

MAX FOOTPRINT = 06'



MARK	DATE	REVISION DESCRIPTION	BY

CDS2025-5-C - 673885-20
OYK MIXED USE
ROCHESTER, MI
for SYSTEM: MH-4

C:\PROJECTS\2025\673885-20\DWG\2025-5-C-673885-20-01.dwg, 3/14/2025 10:14 AM



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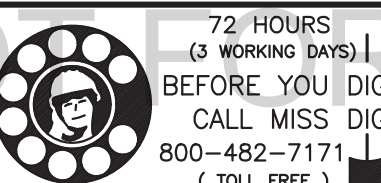
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DATE	CKD.	BY	DATE
3-17-2021			



72 HOURS BEFORE DIG CALL MISS D11 800-482-7171 (TOLL FREE)

Sanitary Calculations & Details
"BEBB OAK MEADOWS" MIXED USE DEVELOPMENT
PART OF THE SE 1/4 OF SECTION 27, T3N, R11E,
CITY OF ROCHESTER HILLS, OAKLAND COUNTY, MICHIGAN

City File #21-008, Section 27
SCALE N/A
SHEET 8 OF 9
KE 2021.053

