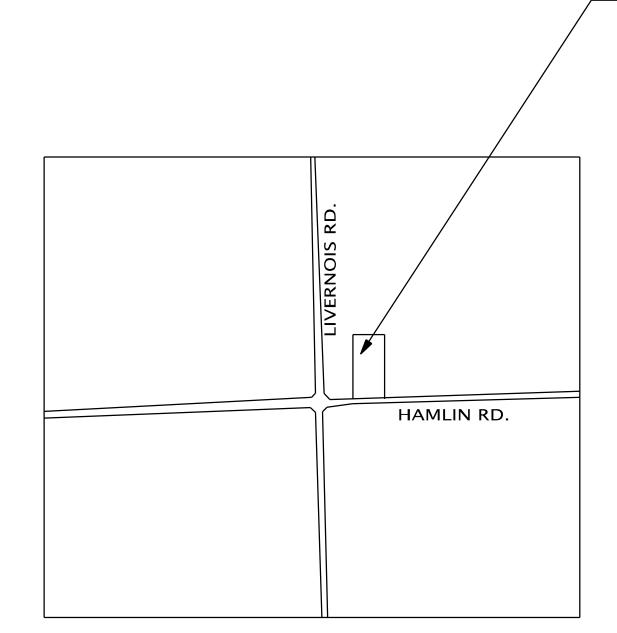
## Proposed Addition

# COVENANT CHRISTIAN CHURCH

900 W. HAMLIN RD. ROCHESTER HILLS, MI

Design Data –	ADDITION				
REFERENCE CODE	MICHIGAN BUILDING CODE (2015 MBC) PLUMBING (2015 MPC) MECHANICAL (2015 MMC) ELECTRICAL (2017 NEC) ICC/ANSI A117.1-2013 ASHRAE 90.1-2013 W/ AMMENDMENTS				
	ENERGY CODE 2015 + ASHRAE 90.1-2013 2017 NEC W/ MICHIGAN PART 8 RULES				
CONSTRUCTION TYPE	VB CONSTRUCTION TYPE (FULLY SPRINKLED) NON-COMBUSTIBLE MATERIALS				
USE GROUP	A-3 ASSEMBLY (PLACE OF WORSHIP/ CHURCH)				
GROSS AREA	SQUARE FOOTAGE OF NEW AND EXISTING				
UNOSS AREA	EXIST. CHURCH LOWER LEVEL: 2,409 Sq.Ft.  UPPER LEVEL: 2,367 Sq.Ft.  Total: 4,776 Sq.Ft.				
	EXIST. CHURCH TOTAL: 4,776 Sq.Ft.  NEW ADDITION 1,489 Sq.Ft.  TOTAL NEW & EXISTING: 6,265 Sq.Ft.				
	NEW PORTICO: 458 Sq.Ft. (UNCONDITIONED)				
ALLOWABLE BUILDING HEIGHT					
PER TABLE 504.3	A- ASSEMBLY AND 5B CONSTRUCTION & (S) SPRINKLED 60 FT. MAX HT. ALLOWABLE 30'-11" HIGHEST PART OF BUILDING				
ALLOWABLE STORIES ABOVE GRADE PER TABLE 504.4	A- ASSEMBLY AND 5B CONSTRUCTION & (S) SPRINKLED ALLOWABLE: 2 STORIES ABOVE GRADE ACTUAL: 1 STORY ABOVE GRADE				
ALLOWABLE AREA PER TABLE 506.2	A- ASSEMBLY AND 5B CONSTRUCTION & (S1) SPRINKLED ALLOWABLE: 24,000 SQ. FT. PER FLOOR				
OCCUPANT LOAD	UPPER LEVEL: SANCTURARY (FIXED SEATING): = 164  LOWER LEVEL: 1,122 SQ. FT (USEABLE)  USED AS CLASS ROOM (20 NET) = 56 OCCUPANTS  UPPER LEVEL NEW: LOBBY SPACE 990 SQ. FT. / 5 NET= 198  (USED FOR EGRESS PURPOSES ONLY)				
	TOTAL OCCUPANT LOAD: 220				
MINIMUM NUMBER OF EXITS FOR OOCUPANT LOAD PER	(2) EXIT REQUIRED- OVER 50 OCCUPANTS (3) EXITS PROVIDED				
MAXIMUM LENGTH OF COMMON PATH EXIT ACCESS TRAVEL	100 FEET ALLOWABLE SPRINKLED NFPA				
PROVIDED	UNDER 100 FEET SPRINKLERED NFPA PROVIDED				
TOTAL EXIT ACCESS TRAVEL DISTANCE	250 FT. MAX DISTANCE (PROVIDED: UNDER 250 FEET)				
IRE-RESISTANCE RATING REQUIREMENTS FOR	R FIRE BARRIER ASSEMBLIES BETWEEN FIRE AREAS				
FULLY SPRINKLERED					
STRUCTURAL FRAME INCLUDING COLUMNS, GIRDERS, TRUSSES	0 HOUR				
BEARING WALLS EXTERIOR INTERIOR	O HOUR EXCEPT AT PROPERTY LINES (2 HOUR) 0 HOUR				
NONBEARING WALLS AND PARTITIONS EXTERIOR	0 HOURS				
NONBEARING WALLS AND PARTITIONSS	0 HOUR				
FLOOR CONSTRUCTION INCLUDING SUPPORTING BEAMS AND JOISTS	0 HOUR				
ROOF CONSTRUCTION					



	DRAWING SET CONTENTS
CS-1	COVER SHEET
D-1	DEMO/ EXISTING PLANS
<b>A</b> -1	PROPOSED FLOOR PLAN
A-2	FRONT & REAR ELEVATIONS
A-3	LEFT & RIGHT ELEVATIONS

CIVIL ENGINEER: MAURO ENGINEERING

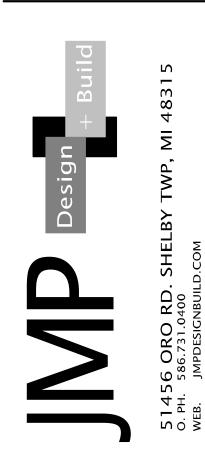
900 W. HAMLIN RD.

48657 Hayes Rd, Shelby Township, MI 48315 Phone: (586) 247-2800

LOCATION MAP



JPIROSKO@JMPDESIGNBUILD.COM



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RCHITECTURAL SEAL

PROJEC

OWENANT CHRISTIAN CHURCI ( ADDITION AND RENOVATION )

BUILDER/CLIENT

COVENANT CHURCH

RELIMINARY

CONSTRUCTION (

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COVER SHEET

DATE: DRAWN BY:

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07-31-2021

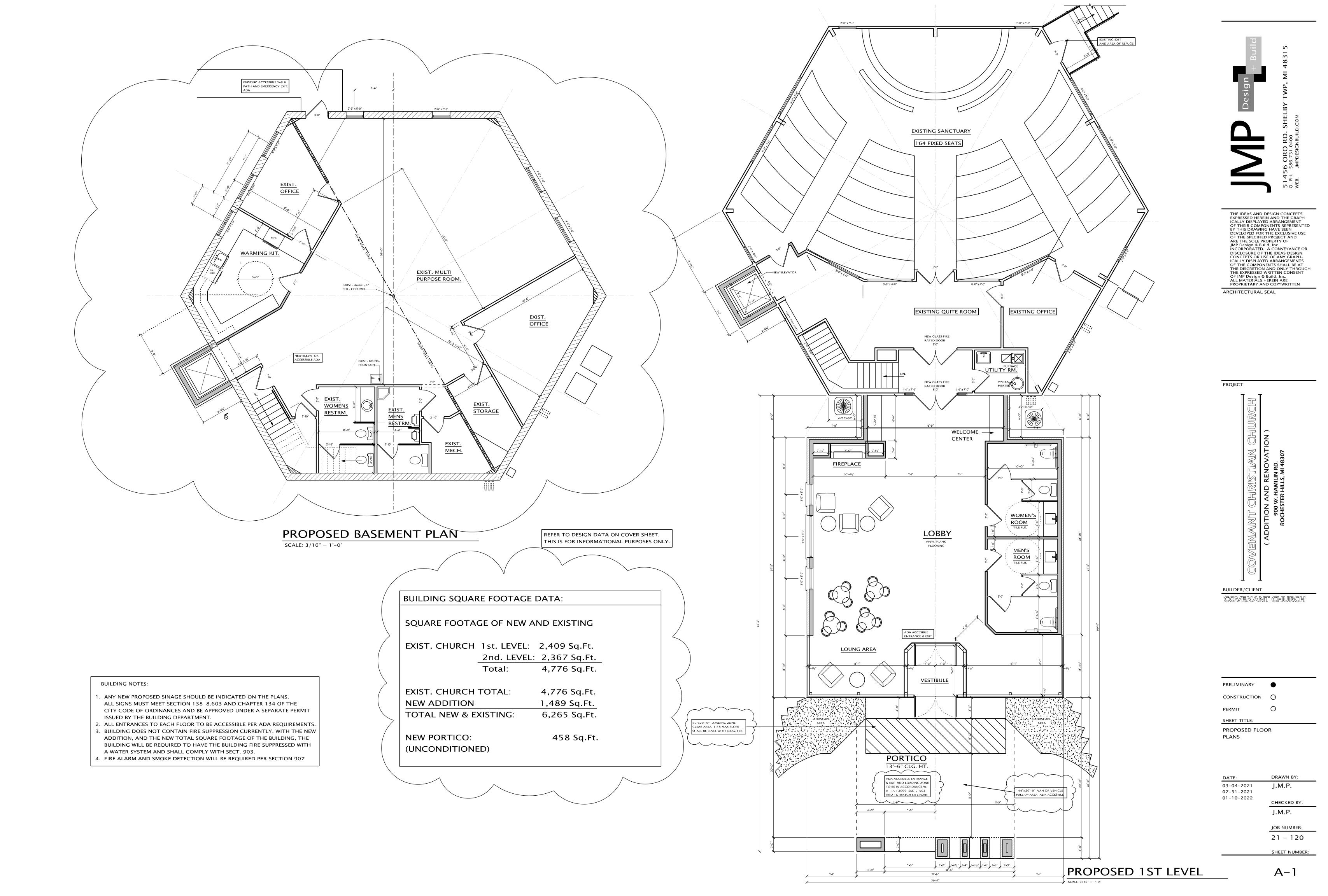
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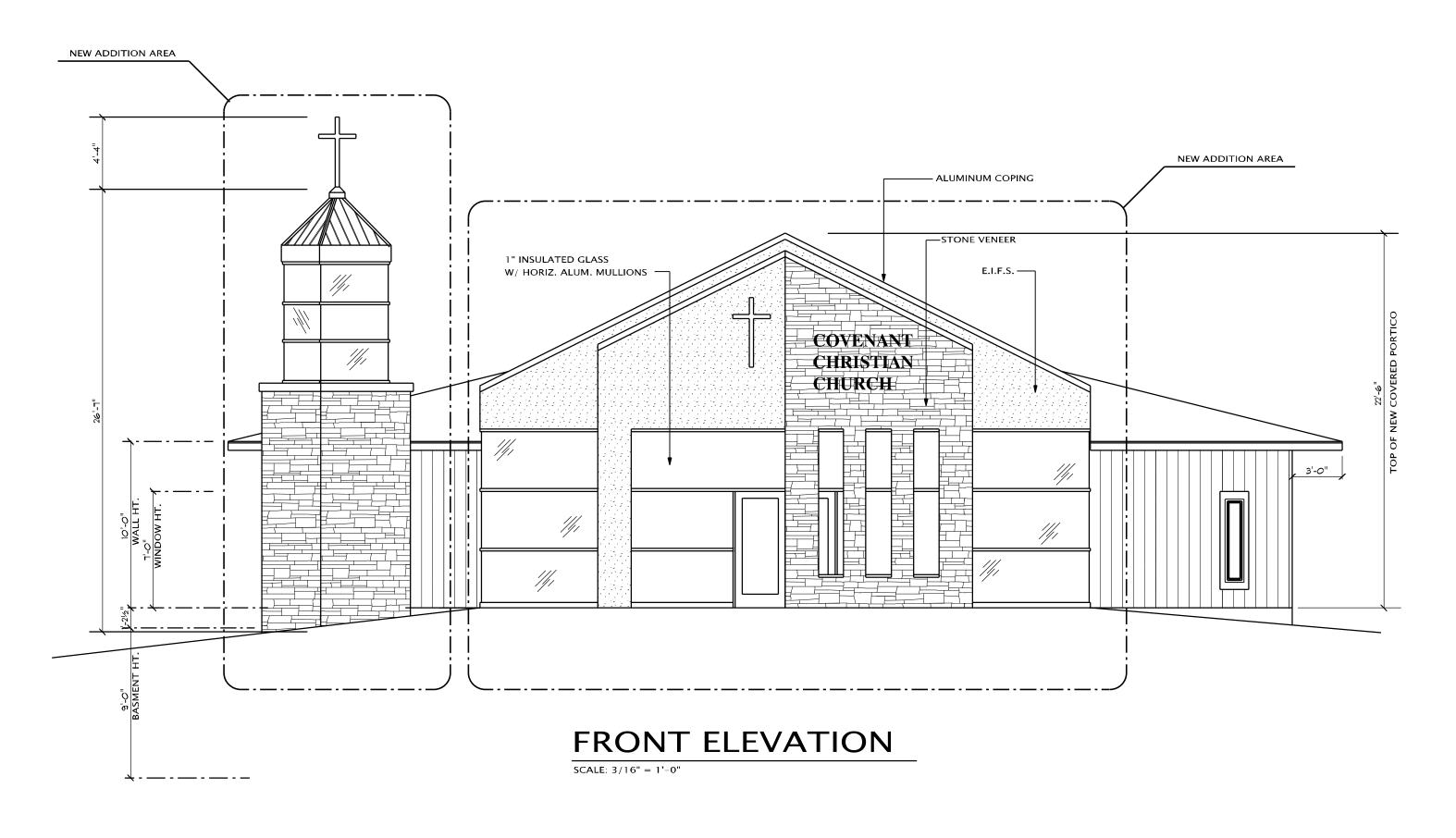
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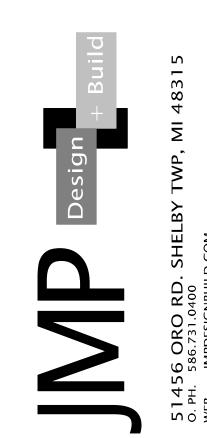
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PROJECT

COWENANT CHRISTIAN CHURCH
( ADDITION AND RENOVATION )
900 W. HAMILIN RD.
ROCHESTER HILLS, MI 48307

BUILDER/CLIENT

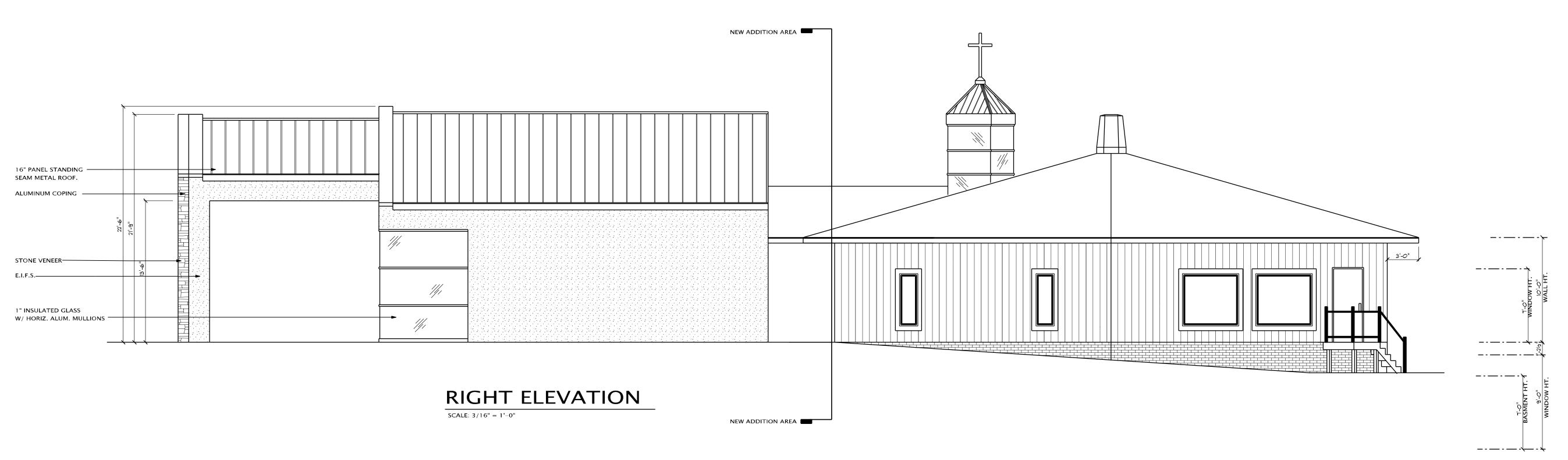
COVENANT CHURCH

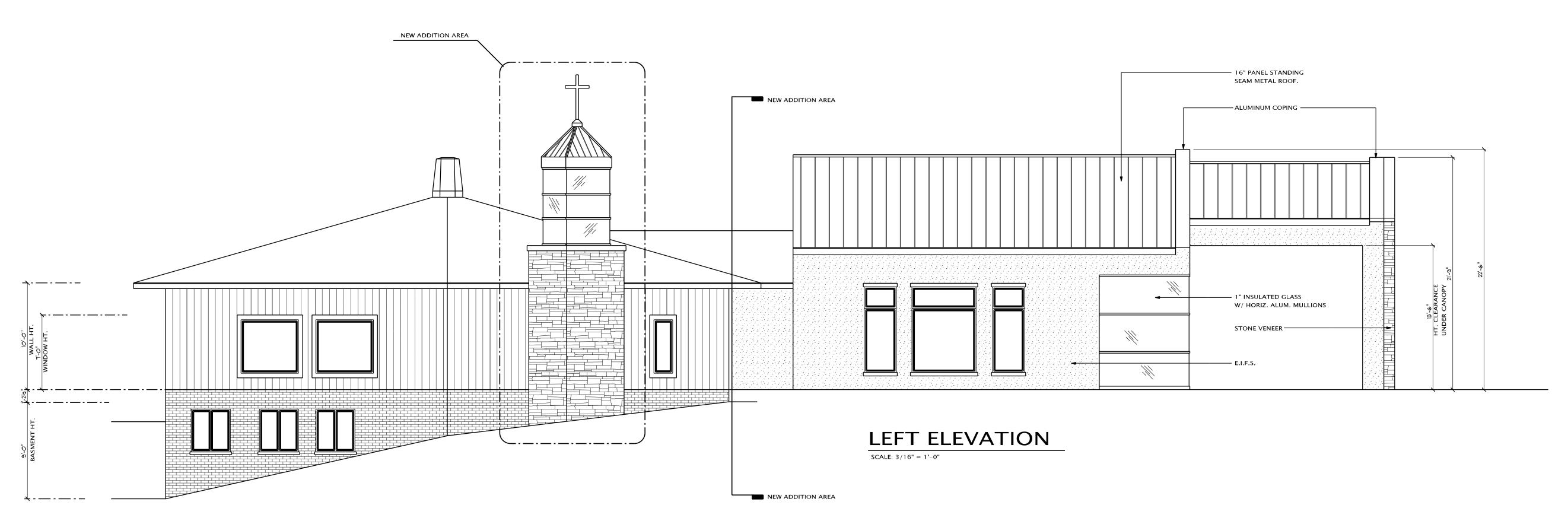
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SHEET TITLE:
FRONT & REAR
ELEVATIONS

A-2





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ARCHITECTURAL SEAL

PROJECT

COWENANT CHRISTIAN CHURCH
( ADDITION AND RENOVATION )
900 W. HAMILIN RD.
ROCHESTER HILLS, MI 48307

BUILDER/CLIENT

COVENANT CHURCH

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LEFT & RIGHT ELEVATIONS

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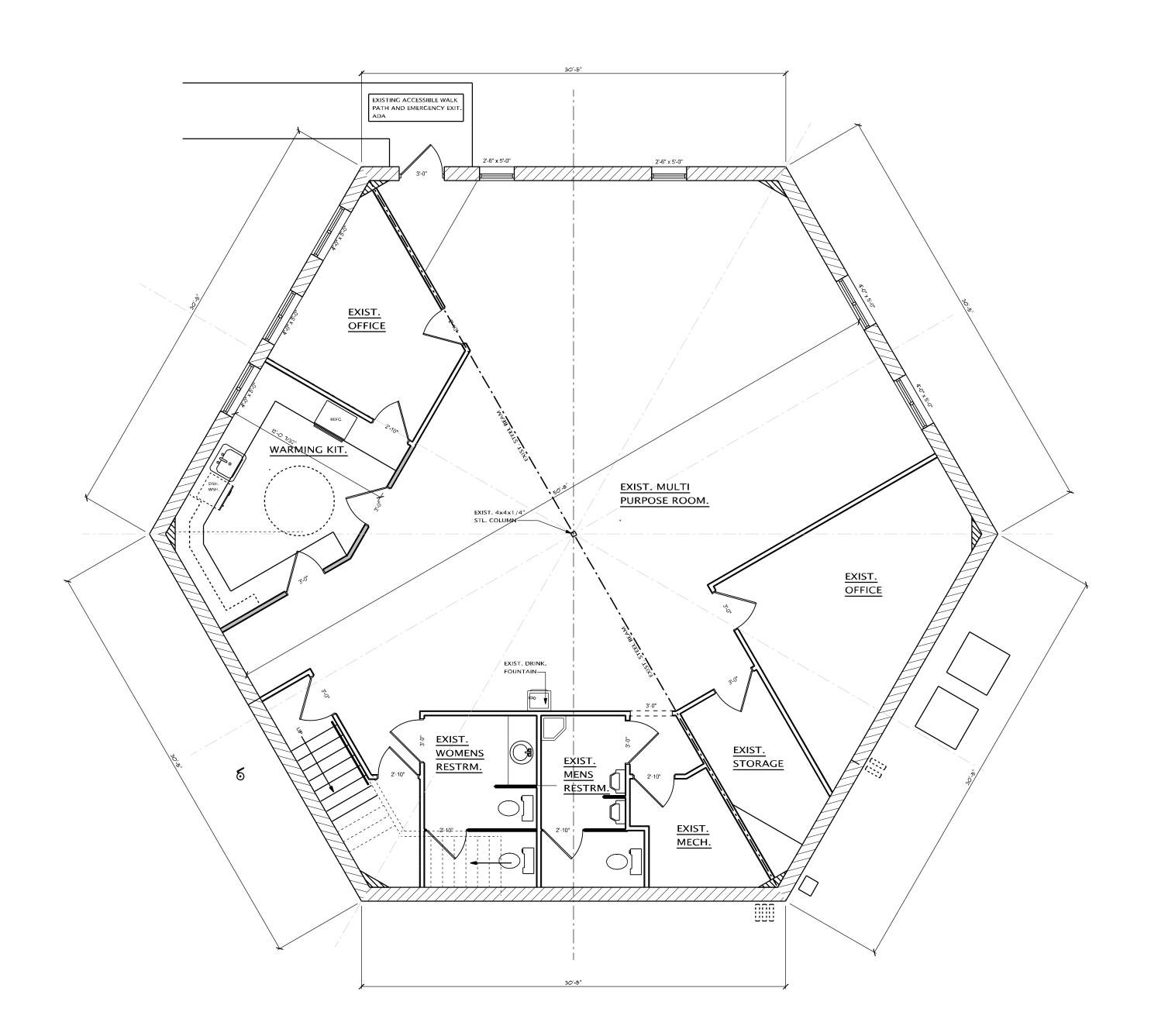
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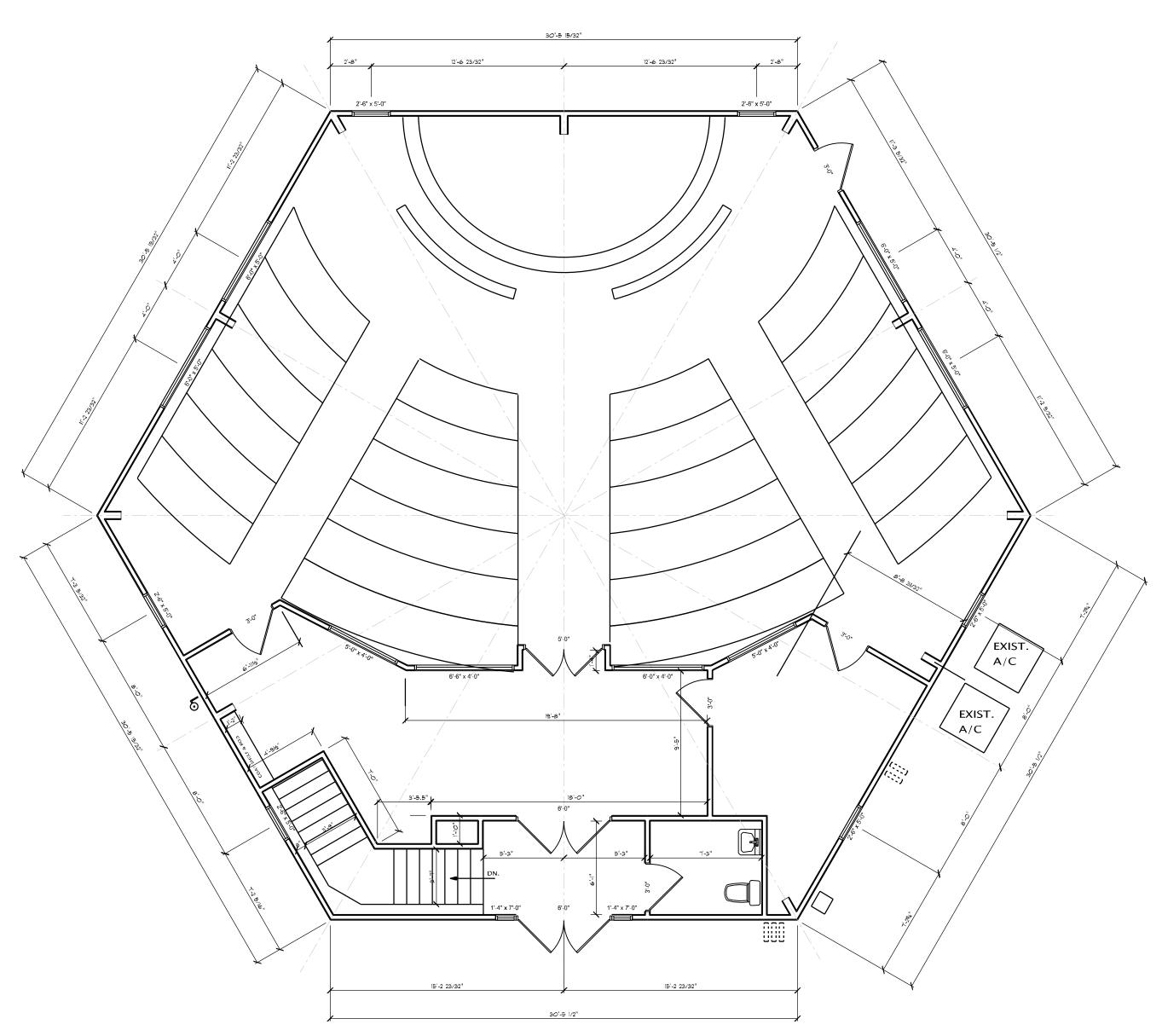
21 - 120

SHEET NUMBER:



EXISTING BASEMENT PLAN

SCALE: 3/16" = 1'-0"



EXISTING MAIN LEVEL

SCALE: 3/16" = 1'-0"

51456 ORO RD. SHELBY TWP, MI 48315 0. PH. 586.731.0400

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ARCHITECTURAL SEAL

COMENANT CHRISTIAN CHURCH
( ADDITION AND RENOVATION )
900 W. HAMILIN RD.
ROCHESTER HILLS, MI 48307

COVENANT CHURCH

PRELIMINARY

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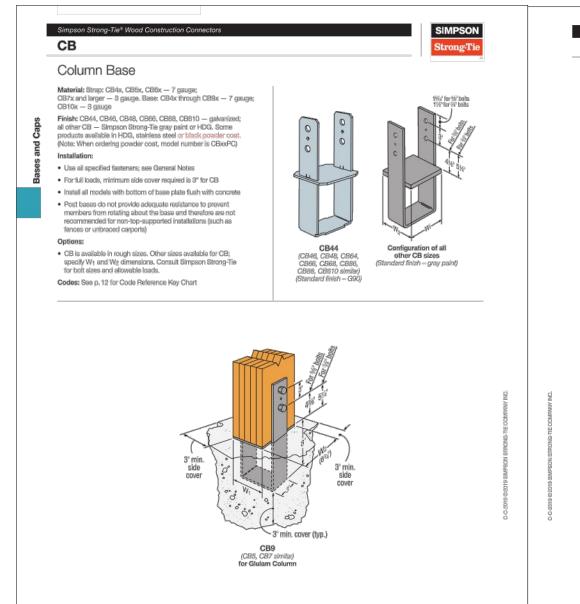
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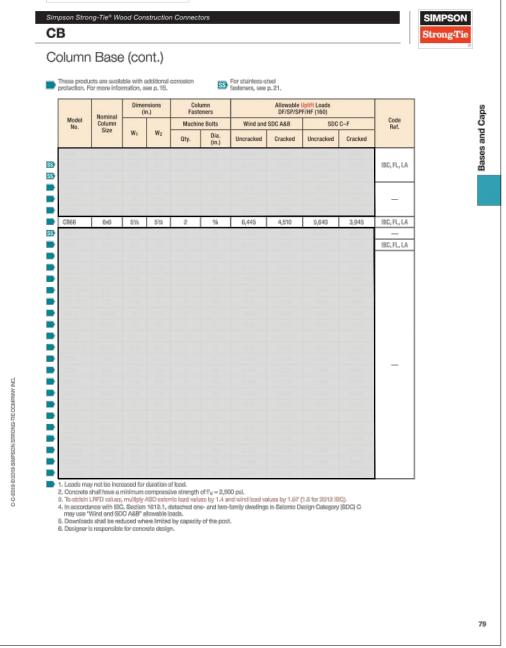
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D-1

## DESIGN DATA

- DESIGN LOADS:
- ROOF DEAD LOAD: 5 PSF, INCLUDING WEIGHT OF ROOF STRUCTURE
- ROOF LIVE LOAD: 20 PSF ROOF SNOW LOAD DATA:
- a. GROUND SNOW LOAD: 30 PSF
- FLAT-ROOF SNOW LOAD, P<sub>f</sub>: 22.7 PSF
   SNOW EXPOSURE FACTOR, C<sub>e</sub>: 0.9
- d. SNOW LOAD IMPORTANCE FACTOR, Is: 1.00
- e. THERMAL FACTOR, Ct: 1.2
- WIND DESIGN DATA:
- a. ULTIMATE DESIGN WIND SPEED (3-SECOND GUST): 115 MPH
- b. NOMINAL DESIGN WIND SPEED, Vasd: 90 MPH
- RISK CATEGORY: II
- d. WIND EXPOSURE: C
- e. INTERNAL PRESSURE COEFFICIENT, GC,: +/-0.18 E.EARTHQUAKE DESIGN DATA:
- a. RISK CATEGORY: II
- b. SEISMIC IMPORTANCE FACTOR, Ie: 1.00
- c. MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS, S1:0.06, SC: 0.193
- d. SITE CLASS: D STIFF SOIL e. DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS, Sps: 0.206, Sp1: 0.095
- f. SEISMIC DESIGN CATEGORY: B
- g. BASIC SEISMIC FORCE-RESISTING SYSTEM: STEEL ORDINARY CONCENTRICALLY BRACED FRAMES
- h. DESIGN BASE SHEAR: 0.06 34W
- i. RESPONSE MODIFICATION COEFFICIENT, R: 3.25
- ANALYSIS PROCEDURE USED: SIMPLIFIED LATERAL FORCE ANALYSIS PROCEDURE
- 2. FOUNDATIONS: SPREAD COLUMN FOOTINGS ARE TO BE DESIGNED FOR 2,000 PSF SOIL BEARING AND TRENCHED FOOTINGS ARE DESIGNED FOR 2,000 PSF. AFTER EXCAVATION, CONTRACTOR SHALL VERIFY BEARING PROVIDED AND, IF NECESSARY, HAVE ENGINEER REDESIGN FOOTINGS BEFORE PLACING CONCRETE. ALL SITEWORK, FOUNDATIONS, DRAINAGE, FILLS ETC. SHALL BE ACCOMPLISHED IN ACCORDANCE WITH STANDARD ENGINEERING PRACTICES. CONCRETE SHALL BE DESIGNED, FURNISHED AND PLACED IN ACCORDANCE WITH THE LATEST ACI SPECIFICATIONS.
- CONCRETE FOR UNEXPOSED FOUNDATION WORK SHALL TEST 3,000 PSI MINIMUM IN STANDARD 6" X 12" CYLINDERS AT 28 DAYS. CONCRETE FOR FLOORS AND SLABS SHALL TEST 3,500 PSI. EXTERIOR CONCRETE SHALL HAVE 6% (+/- 1%) ENTRAINED AIR AND SHALL TEST 4,000 PSI MINIMUM.
- REINFORCING STEEL SHALL BE ASTM A615, GRADE 60, DESIGNED, DETAILED, FABRICATED AND ERECTED ACCORDING TO THE LATEST ACIAND CRS1 SPECIFICATIONS.
- HOOK ALL FOOTING REBAR 1'-6" AROUND CORNERS OR PROVIDE CORNER BARS.
- NO RISES, CONDUITS, ETC. WITHIN 1'-4" OF CENTERLINE OF LOAD CONCENTRATION.
- E.ALL SLABS SHALL BE REINFORCED AND SHALL HAVE SAWN CONTROL JOINTS 1/4 OF THE THICKNESS OF THE SLAB, AS SHOWN ON THE DRAWINGS. F.ANCHOR ROD DESIGN AND FABRICATION SHALL BE AS REQUIRED UNDER ASTM F1554, LATEST EDITION. ANCOR RODS IN CONTACT WITH
- PRESSURE-TREATED WOOD SHALL BE STAINLESS STEEL. 4. STEEL STRUCTURE SHALL BE DESIGNED, DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST MBMA AND AISC SPECIFICATIONS.
- ALL STEEL SHALL BE ASTM 572 (50 KSI).
- ALL WELDS SHALL BE WITH E70XX ELECTRODES AND PER AWS REQUIREMENTS.
- STRUCTURAL BOLTS SHALL BE A325.
- STEEL SHALL HAVE ONE COAT OF RED CHORMATE PAINT, SHOP APPLIED. PARTS INACCESIBLE AFTER ERECTION SHALL HAVE TWO COATS. E.CONNECTIONS SHAL DEVELOP FULL STRENGTH OF MEMBERS.
- F.PURLINS, ROOF COVERING, AND WALL COVERING MATERIAL SHALL BE MANUFACTURED AND FURNISHED BY NORTH STAR METALS MFG., 6850 EDWARDS RIDGE RD. SE, UHRICHSVILLE, OH 44683. RAKE BEAMS AND ENDWALL COLUMNS TO BE AS NOTED ON PLANS. ROOF PANELS SHALL BE 26 OR 29 GAUGE MAXRIB (80 KSI) AND WALL PANELS SHALL BE 26 OR 29 GAUGE MAXRIB (80 KSI). PANEL FASTENERS SHALL BE SELF-DRILLING SCREWS WITH HEX
- HEADS (PAINTED) WITH NEOPREME WASHERS. STEEL ROOF AND WALL BRACING, AS REQUIRED, SHALL BE DESIGNED AND INSTALLED TO MEET DESIGN WIND LOADS. 5. ALL WOOD PURLINS, GIRTS AND DOOR FAMING SHALL BE NO. 1 SYP. ALL WOOD IN CONTACT WITH CONCRETE SHALL BE "PRESSURE-TREATED". ALL
- FASTENERS AND CONNECTORS IN CONTACT WITH PRESSURE-TREATED WOOD SHALL BE STAINLESS STEEL.
- 6. ALLOWABLE STRESS INCREASES WILL BE PERMITTED FOR WOOD; 15% FOR SHORT-TERM SNOW LOADING AND 60% FOR SHORT-TERM WIND LOADING.
- 7. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER IT IS FULLY ERECTED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE THE ERECTION PROCEDURES AND SEQUENCE AND TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION.
- 8. RESPONSIBILITY TO CONFORM TO THE ENGINEER'S DRAWINGS AND SPECIFICATIONS IS THAT OF THE CONTRACTOR. THE ENGINEER CERTIFIES THAT THE DESIGN IS IN ACCORDANCE WITH THE CURRENT APPLICABLE CODES AND SPECIFICATIONS, BUT TAKES NO RESPONSIBILITY OF THE CONTRACTOR'S FAILURE TO
- SAFETY FACTORS HAVE BEEN INCLUDED IN THE DESIGN OF ALL MEMBERS.





## **OWNER**

## Pastor Clay Bernier

Covenant Christian Church Rochester Hills, MI 48307

#### NOTE:

CONSTRUCTION DOCUMENTS AT PROJECT SITE. AN APPROVED SET OF CONSTRUCTION DOCUMENTS SHALL BE KEPT AT THE SITE OF THE WORK AND SHALL BE AVAILABLE FOR REFERENCE BY BUILDING OFFICIAL OR THE BUILDING OFFICIALS DESIGNATED REPRESENTATIVE AT ALL TIMES DURING WORKING HOURS WHILE SUCH WORK IS IN PROGRESS.

## MICHIGAN CODE

PROJECT NO. 2021-32

M.B.C. 2015 MICH ELECTRICAL CODE 2017 INTERNATIONAL FIRE CODE 2015 **BUILDING USE:** (A-1) ASSEMBLY.

CONSTRUCTION TYPE: V-B

OCCUPANCY LOAD: 80 PEOPLE

**ALLOWABLE AREA:** 5,500 SF. 1 STORY

PROPOSED: 1,200 SF 1 STORY

## **ARCHITECT**

ORIN E. CAMPBELL ARCHT. # 1301052654 CAMPBELL ASSOCIATES, INC. 550 E. FRENCHLINE RD. SANDUSKY, MI. 48471 810.648.5099





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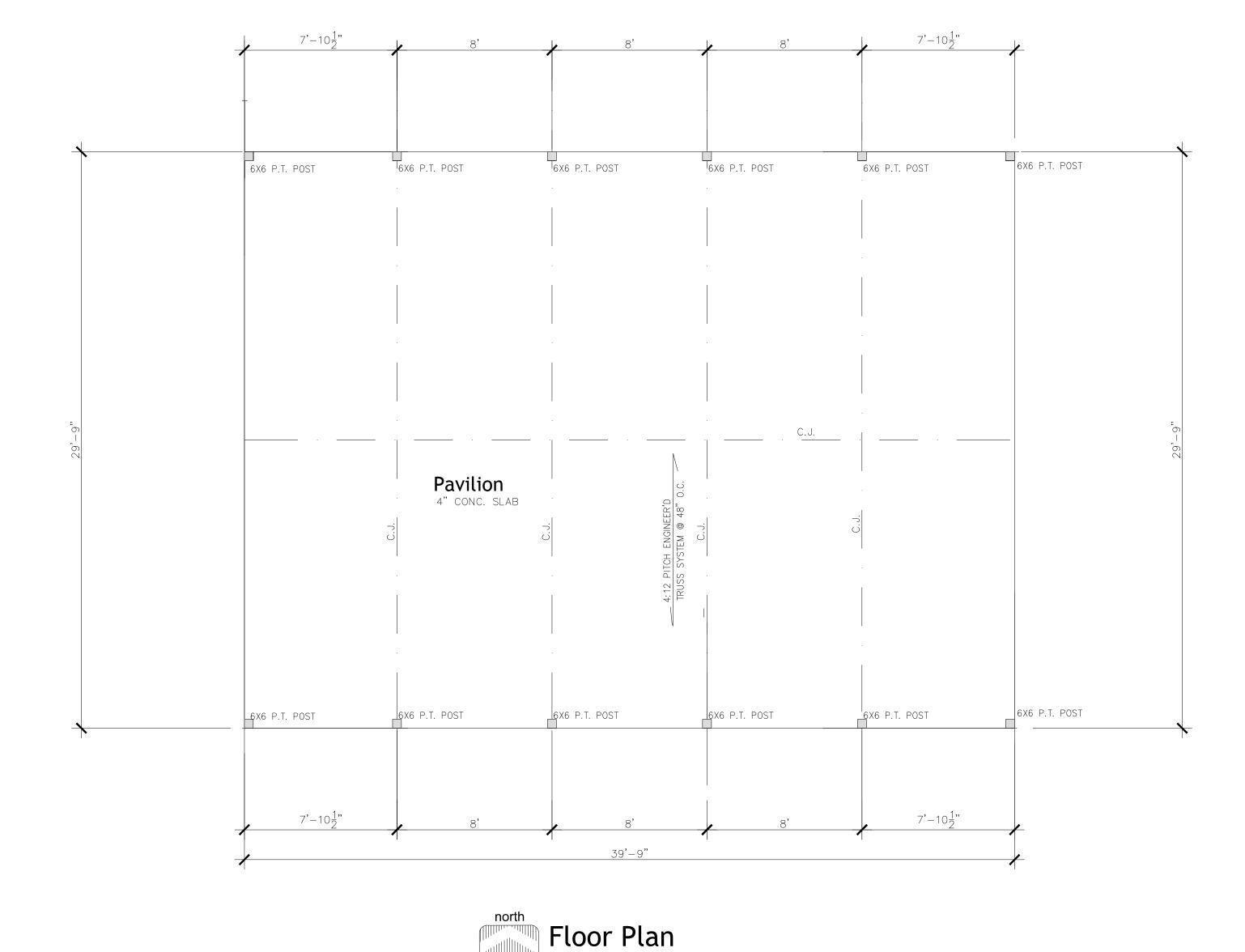
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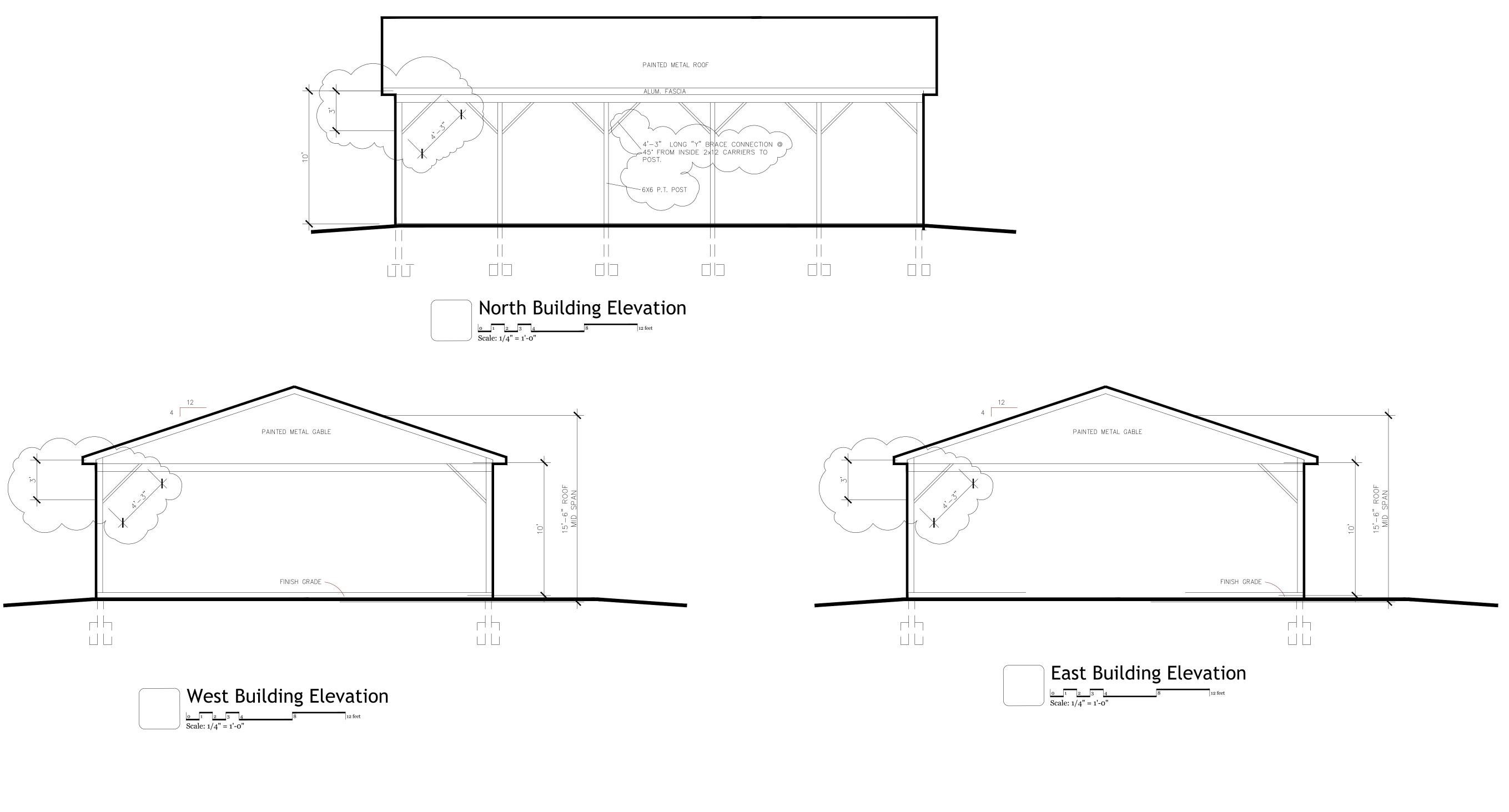
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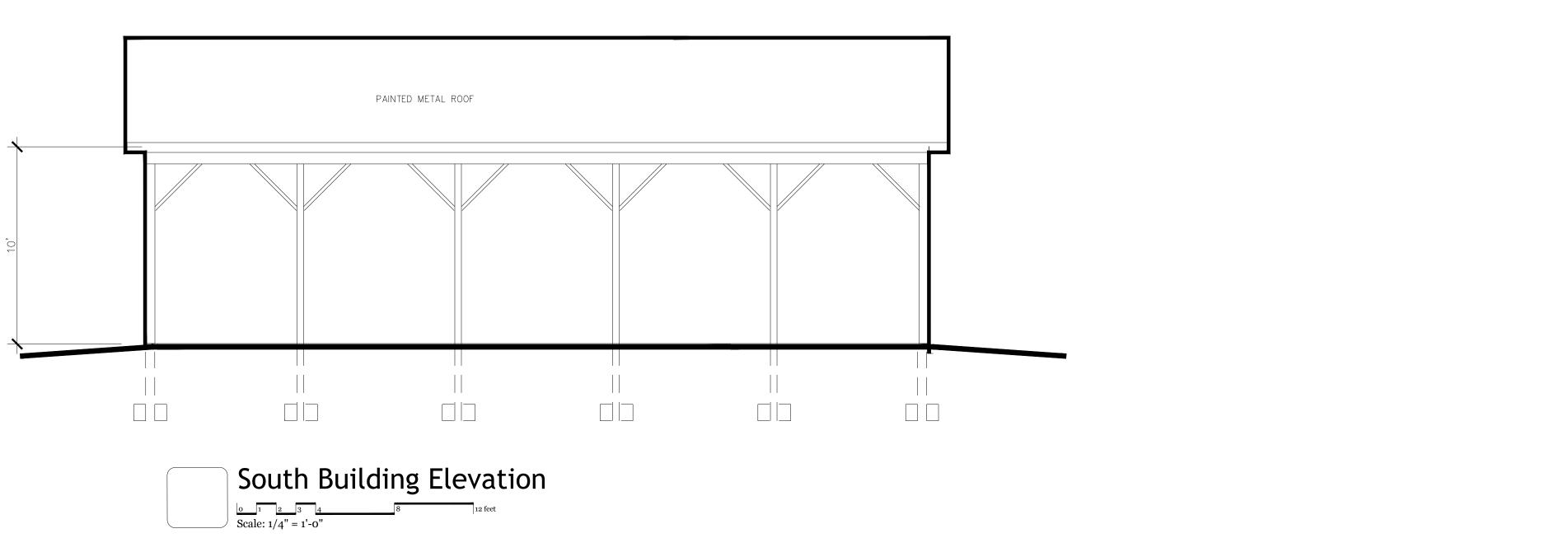
associates, i engineers planr

**Sheet Number:** 



o 1 2 3 4 Scale: 1/4" = 1'-0"







Covenant Christian Churc

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Every effort has been made to ensure the accuracy of these plans. However, it is the responsibility of the Contractor to review and verify all dimensions and structure prior to the start of construction.

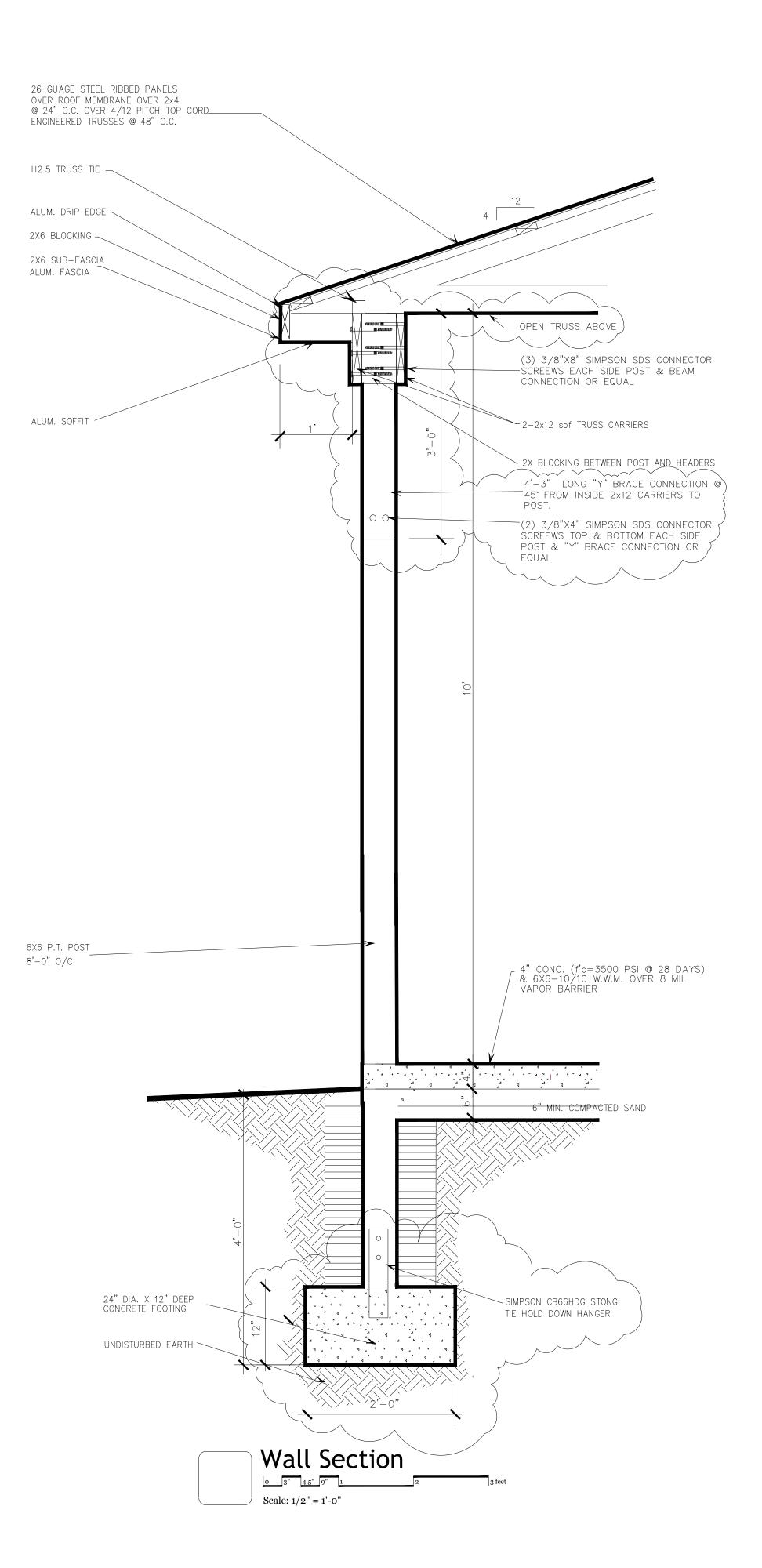
Notify the Architect Immediately where concerns or discrepancies may occur.

Do Not scale this print.

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	Date	15April2021	26Jan2022				
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Project No: 2021-32
Drawn by: 0EC
Date: 15April2021
Sheet Contents:
Building Elevations

a2
Sheet Number:



#### SUBGRADE PREPARATION:

THE TOPSOIL, FILL SOILS, ORGANIC SOILS, SOFT SOILS AND ANY OTHER UNSUITABLE MATERIALS SHALL BE REMOVED FORM THE BUILDING AND PARKING AREAS PRIOR TO PLACEMENT OF ENGINEERED FILL. THE RESULTING GRADE SHOULD THEN BE PROOF ROLLED WITH A LOADED TANDEM TRUCK OR FRONT LOADER TO DEVELOP ANY WEAK AREAS IN THE GRADE. THE SOIL IN ANY WEAK AREA SHOULD BE REMOVED AND REPLACED WITH DRIER SIMILAR SOILS AND PROOF ROLLED TO PROVEN COMPACTION.

PRIOR TO PLACEMENT OF ANY FILL, THE SUBGRADE SHOULD BE STEPPED AND BENCHED. ANY FILL PLACED IN THE BUILDING OR PARKING AREAS SHOULD BE AN APPROVED MATERIAL FREE OF FROZEN SOIL, ORGANIC OR OTHER DELETERIOUS MATERIAL. THE SOILS ENCOUNTERED IN THE BORINGS APPEAR TO BE SUITABLE FOR RE-USE AS ENGINEERED FILL. CLAY SOILS USED AS FILL SHOULD BE PLACED WITHIN 2 % OF THEIR OPTIMUM MOISTURE CONTENT. THEY WILL REQUIRE THE USE OF SHEEP-FOOT COMPACTORS AND IT MAY ALSO BE NECESSARY TO USE A DISC TO AERATE SOILS WITH MOISTURE CONTENTS THAT ARE OVER OPTIMUM. IT SHOULD BE NOTED THAT IT WILL BE DIFFICULT TO CONTROL THE MOISTURE CONTENT IN CLAY SOILS DURING COLD AND/OR WET WEATHER CONDITIONS. IF FILL IS TO BE PLACED THIS WINTER, CLAY SOIL SHOULD NOT BE USED.

IT IS RECOMMENDED THAT ANY ADDITIONAL FILL SOIL BROUGHT IN FROM OFF THE SITE CONSIST OF A GRANULAR MATERIAL MEETING THE GRADATION REQUIREMENTS FOR MDOT CLASS II BACKFILL.

FILL SOILS SHOULD BE PLACED IN 9" LOOSE LIFTS AND UNIFORMLY COMPACTED TO A T LEAST 95.0 MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-1557(MODIFIED PROCTOR). ENGINEERED FILL SHOULD BE

#### **MASONRY**

CONCRETE MASONRY PIERS AND ERECTION SHALL CONFORM TO SPECIFICATIONS FOR DESIGN AND CONSTRUCTION OF LOAD BEARING CONCRETE MASONRY BY 'NATIONAL CONCRETE ASSOCIATION' AND ASTM C90 WITH A MINIMUM COMPRESSIVE PRISM STRENGTH OF 1800 PSI.

MORTAR FOR CONCRETE AND MASONRY SHALL CONFORM TO ASTM C270 TYPE M WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI FOR BELOW GRADE APPLICATIONS AND TYPE S MORTAR WITH A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI FOR ABOVE GRADE APPLICATIONS.

GROUT TO FILL CORES OF PIERS WHERE INDICATED SHALL CONFORM TO ASTM C476 WITH MINIMUM COMPRESSIVE CYLINDER STRENGTH OF 2500 PSI AT 28 DAYS. GROUT SHALL BE RODDED AND OR VIBRATED TO INSURE COMPLETE FILLING OF THE

HORIZONTAL JOINT REINFORCEMENT SHALL BE TRUSS TYPE. IT SHALL BE PLACED AT 16" ON CENTER IN ALL PIERS. WIRES SHALL BE 9 GAGE CONFORMING TO ASTM A82. PREFABRICATED CORNER SECTIONS SHALL BE USED AT ALL MASONRY WALL CORNERS.

NO MASONRY SHALL BE LAID WHEN OUTSIDE TEMPERATURE IS BELOW 40 DEGREES FAHRENHEIT UNLESS METHODS APPROVED BY ENGINEER ARE USED DURING CONSTRUCTION TO PREVENT DAMAGE TO THE MASONRY.

#### **SLABS**

STRUCTURAL SLABS AND THICKENED FLOORS SHALL BE PLACED IN ACCORDANCE WITH THE PLANS.

PROVIDE W1.4 X W1.4 6X6 WWF REINFORCEMENT UNLESS INDICATED OTHERWISE ON THE DRAWINGS. PLACE FABRIC A MINIMUM OF 1 1/2' AND A MAXIMUM OF 2" BELOW THE TOP SLAB. OVERLAP LENGTH FOR THE SPLICES SHALL BE NO LESS THAN 6" MEASURED BETWEEN THE OUTERMOST INTERCONNECTED ACCORDING TO THE DRAWINGS. CROSS WIRES OF EACH FABRIC SHEET.

CONCRETE AND STEEL CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3,000 P.S.I.

REINFORCING BARS: ASTM A615 GRADE 60, DEFORMED.

WELDED WIRE FABRIC: ASTM A185 OR USE

AS PER CURRENT 'ACI' CODES.

CONCRETE w/ FIBERMESH INSTALL AND COVER ALL CONCRETE REINFORCEMENT

HOT ROLLED SHAPED TO BE ASTM A36; TUBE SECTIONS TO BE ASTM A500 GRADE B.

#### **GENERAL**

THE STRUCTURAL STEEL, MASONRY, CONCRETE, AND WOOD WALLS HAVE BEEN DESIGNED TO RELY UPON THE ROOF AND FLOOR SYSTEM FOR STABILITY. CONTRACTOR SHALL PROVIDE ADEQUATE TEMPORARY BRACING FOR ALL STRUCTURAL STEEL AND MASONRY WALLS UNTIL PERMANENT BRACING, AND FLOOR SLABS HAVE BEEN INSTALLED AND THESE ELEMENTS ARE

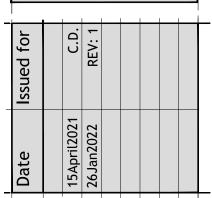
#### **GEOTECHNICAL NOTE:**

1. THE FOUNDATIONS AND BUILDING SLABS FOR THIS SITE WILL BEAR ON NATIVE SOILS OR ENGINEERED FILL. A PROFESSIONAL GEOTECHNICAL ENGINEER OR HIS ACCREDITED REPRESENTATIVE SHALL VERIFY ALL BEARING CAPACITIES PRIOR TO PLACEMENT OF FOUNDATIONS OR SLABS. A PROFESSIONAL GEOTECHNICAL ENGINEER OR HIS ACCREDITED REPRESENTATIVE SHALL VERIFY ALL FILLING OPERATIONS, SUCH THAT THE ENGINEERED FILL DOES NOT CONTAIN FROZEN OR UNSUITABLE MATERIAL, THAT IT MEETS THE SPECIFICATION OF MDOT CLASS II, AND IS PLACED AND COMPACTED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE PLANS.





Chur Christian Covenant



**Sheet Number:** 

