# **CAMDEN CROSSING**

9.36 ACRE DEVELOPMENT CITY OF ROCHESTER HILLS, OAKLAND COUNTY, MICHIGAN

## PRELIMINARY SITE PLAN

## **DEVELOPMENT TEAM**

## DEVELOPER

M2J1, LLC 14955 TECHNOLOGY DRIVE SHELBY TOWNSHIP, MI 48315 PHONE: (586) 421-5729 CONTACT: JIM POLYZOIS

## **CIVIL ENGINEER**

ATWELL, LLC TWO TOWNE SQUARE, SUITE 700 SOUTHFIELD, MI 48076 PHONE: (248) 447-2000 CONTACT: ERIC LORD, P.E.

## LANDSCAPE ARCHITECT NUNEZ DESIGN, INC 249 PARK STREET TROY. MI 48083

PHONE: (248) 224-5933 CONTACT: RALPH NUNEZ

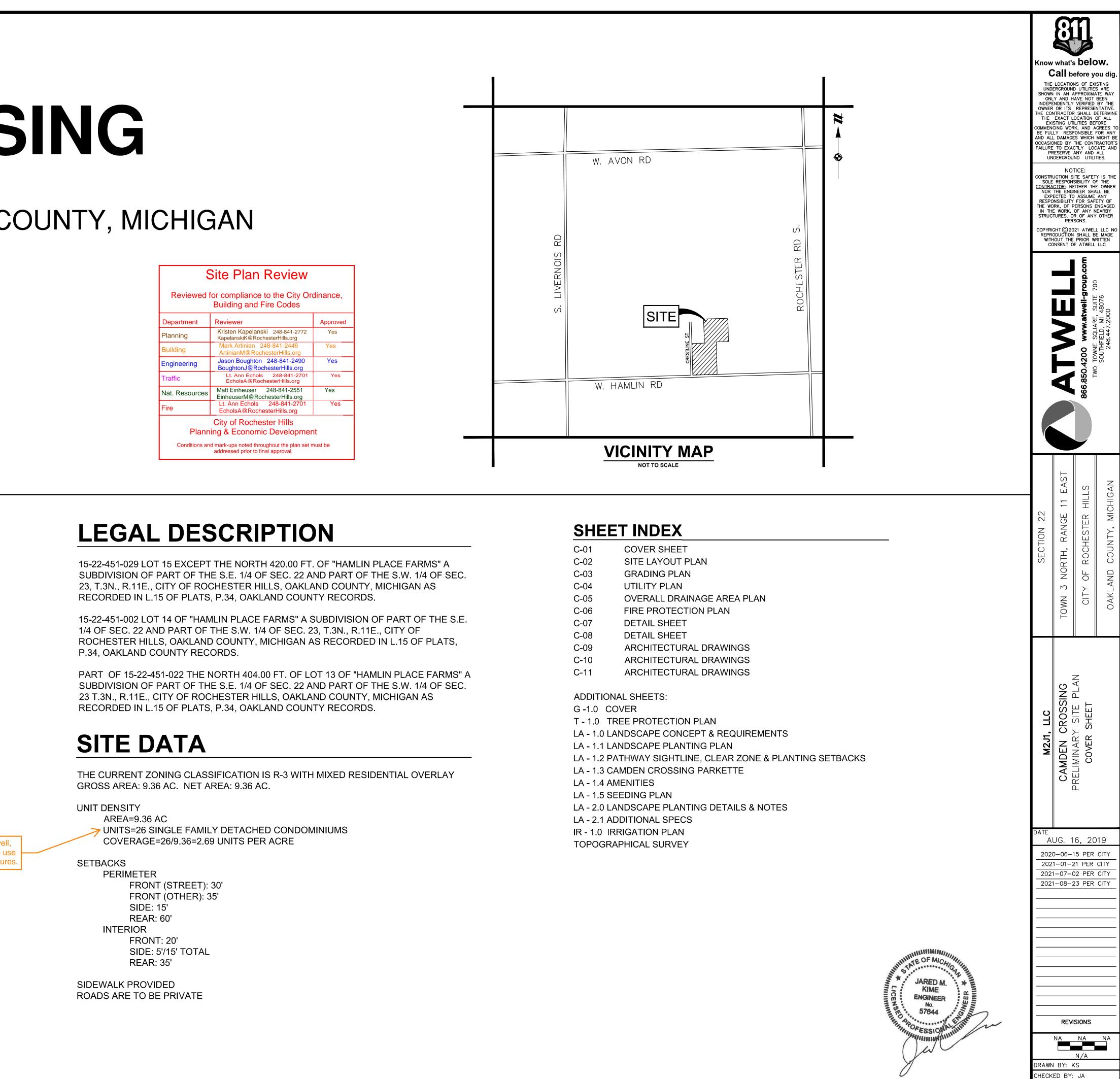
## SURVEYOR

REICHERT 140 FLUMERFELT LANE ROCHESTER, MI 48306 PHONE: (248) 651-0592 CONTACT: GEORGE REICHERT

Minimum site area using the MR Overlay is 10 acres. The Planning Commission may modify this requirement if another standard is more reasonable due to existing site or neighborhood conditions or because the site cannot physically comply with one or more of the requirements. In determining whether the modification is warranted, the Planning Commission shall consider the findings for approving a conditional use listed in Section 138-2.302 of the Zoning Ordinance.

> Per a phone conversation with John Ackerman of Atwell, LLC on 6/29/2020, the Design Professional intends to use the 2015 Michigan Residential Code for all new structures.

Colored renderings must be provided prior to the Planning Commission meeting.

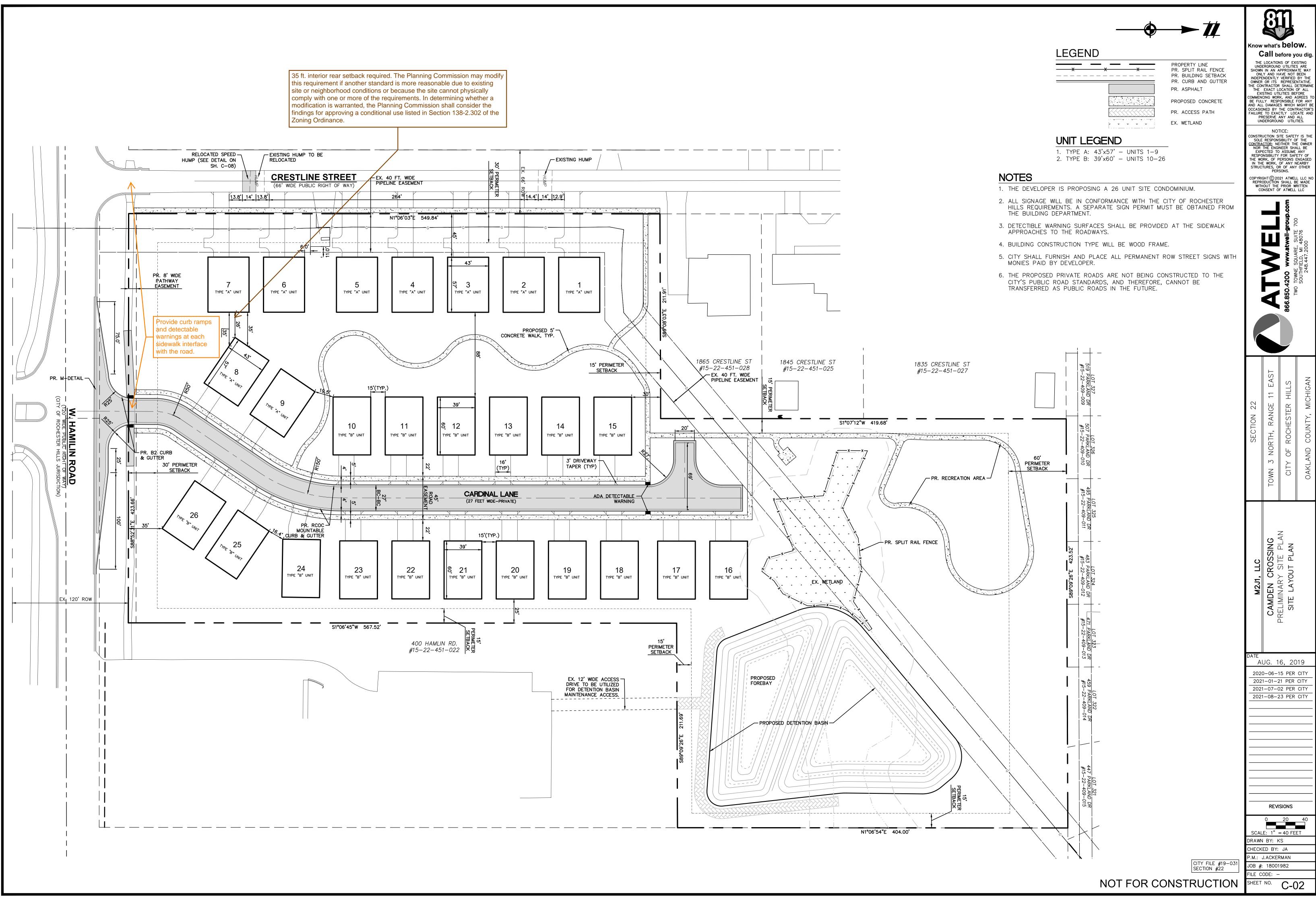


.M.: J.ACKERMAN

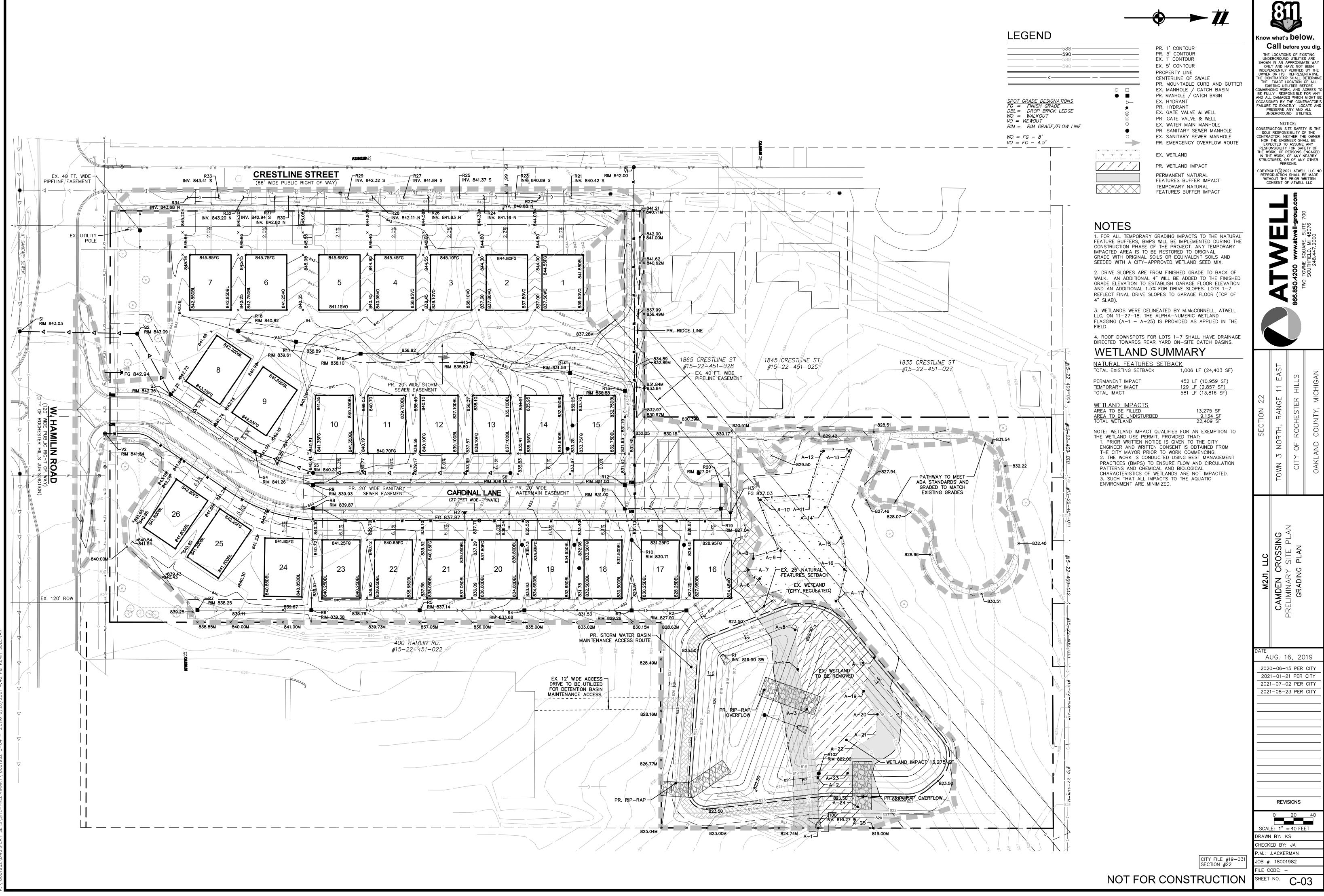
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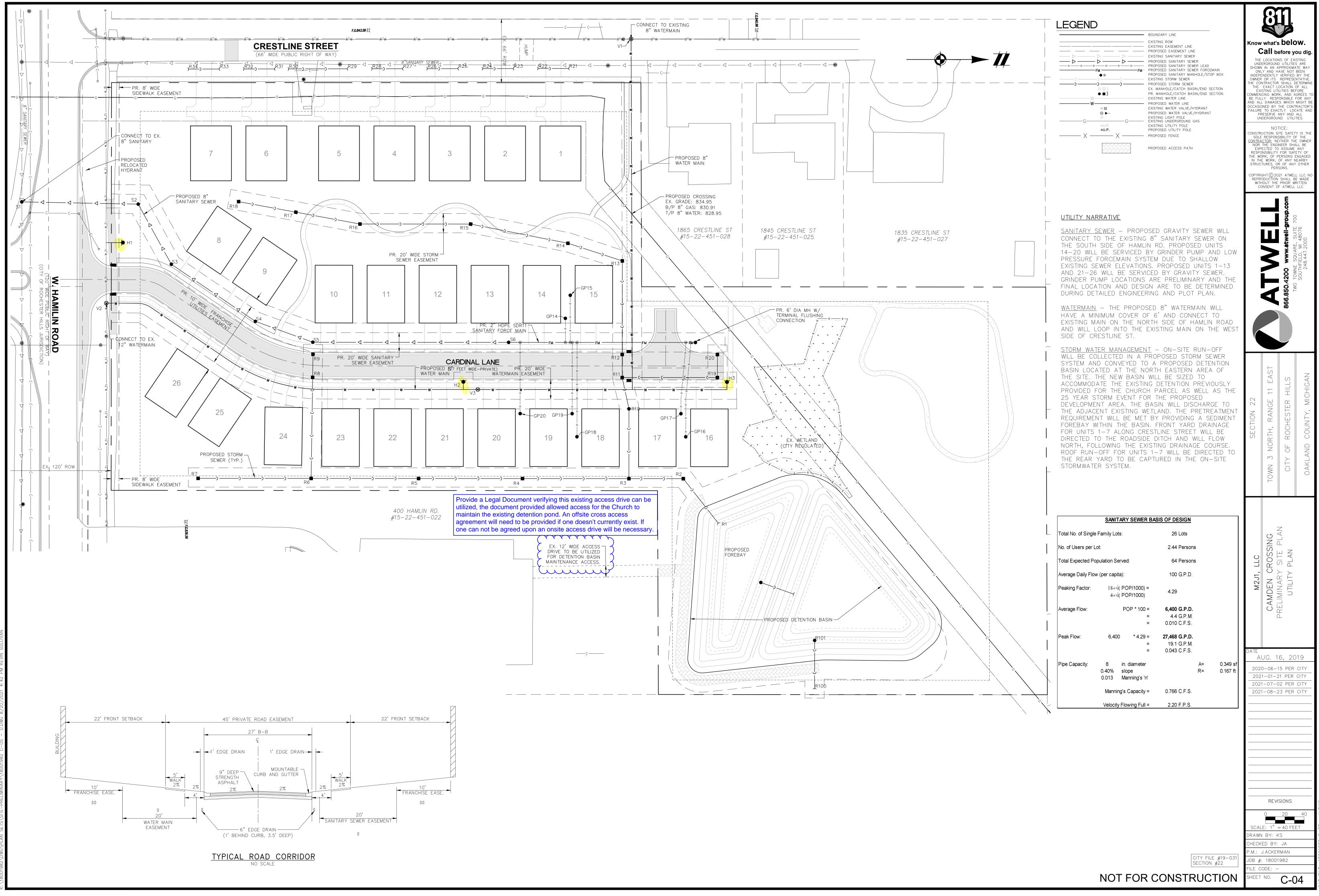
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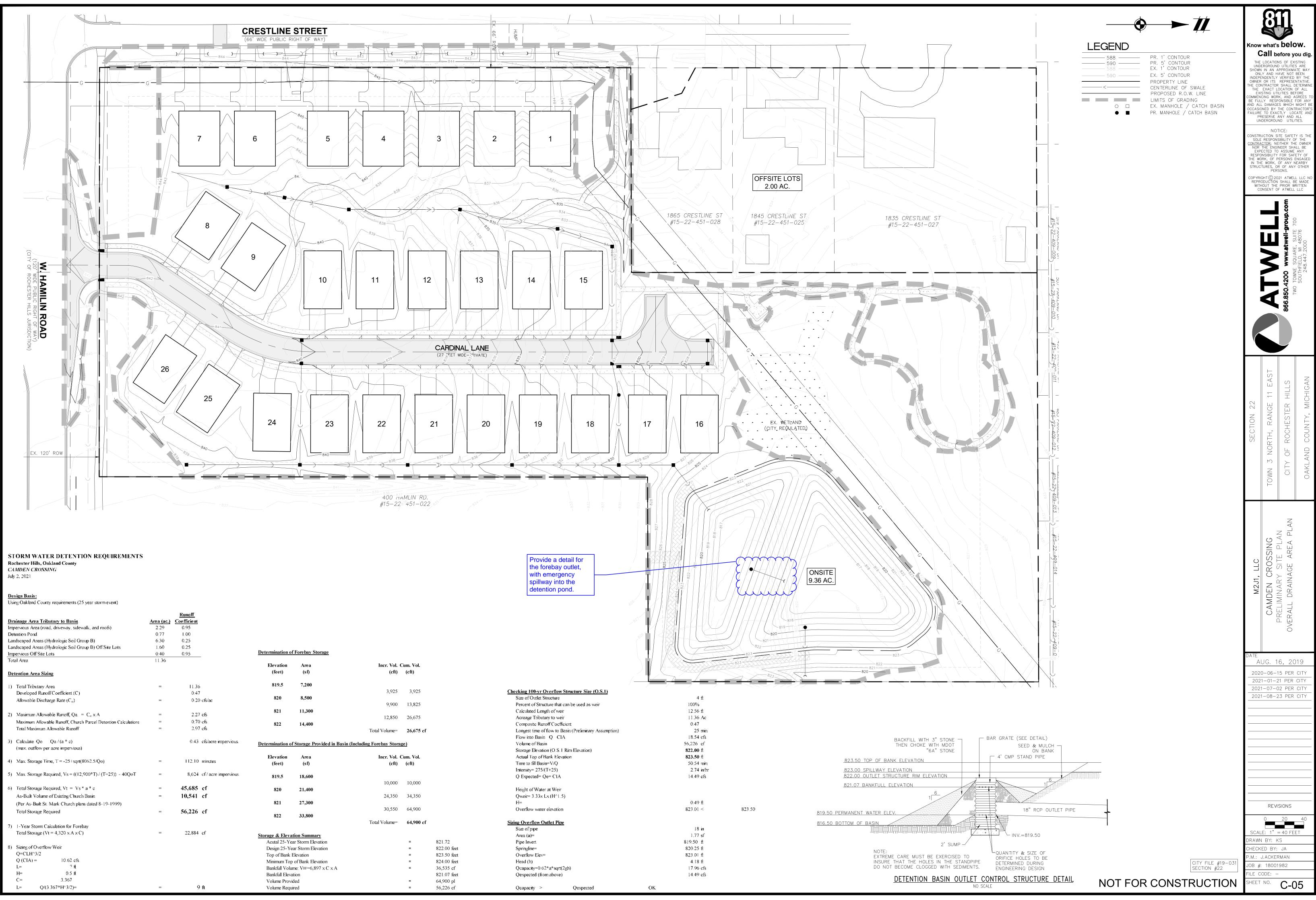
CITY FILE #19-031 SECTION #22 NOT FOR CONSTRUCTION SHEET NO.



01982\DWG\PLAN SETS\SITE-PRELIMINARY\18001982 C-03 - L.DWG 8/20/2021 4:41 PM KEVIN SULLIVAN





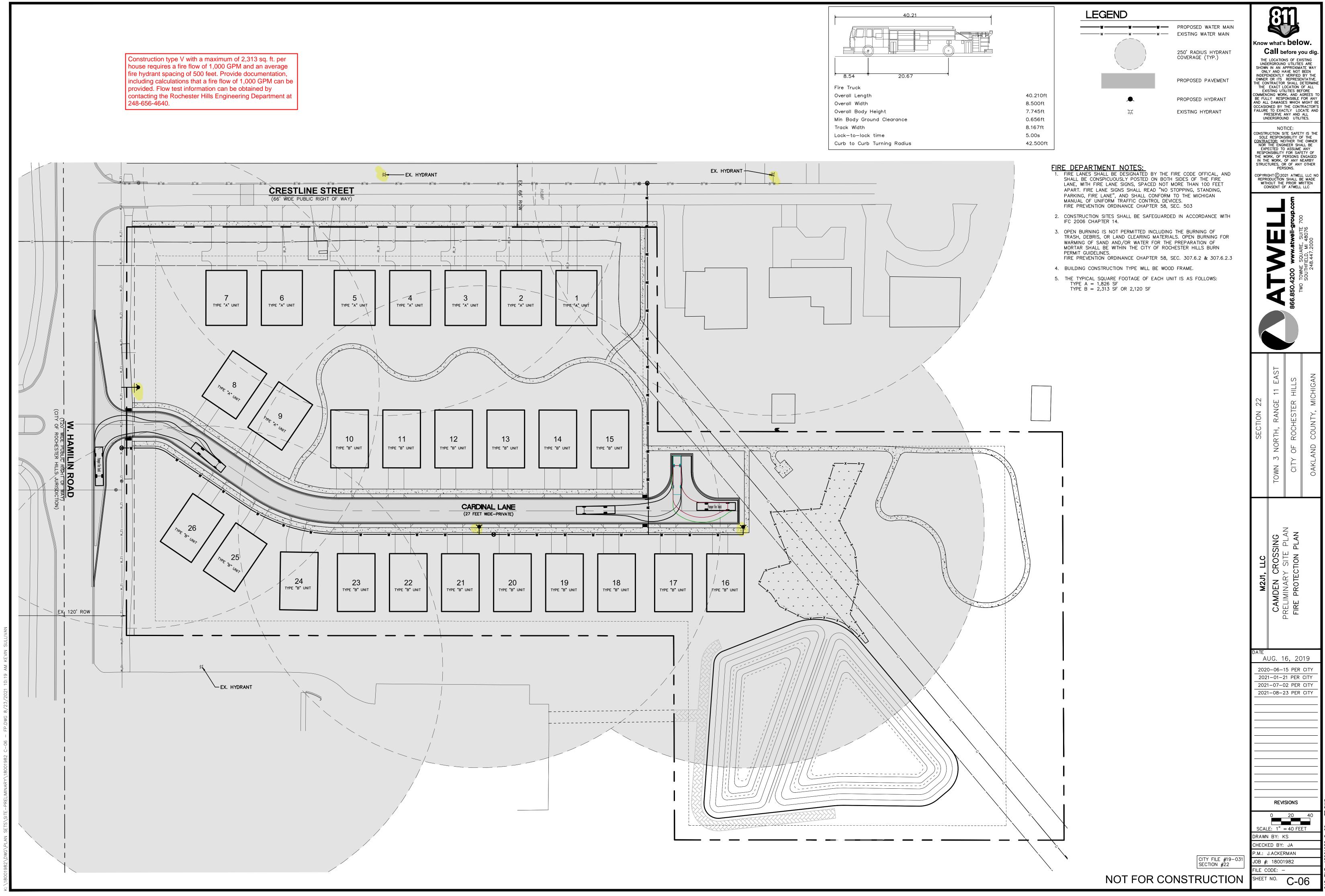


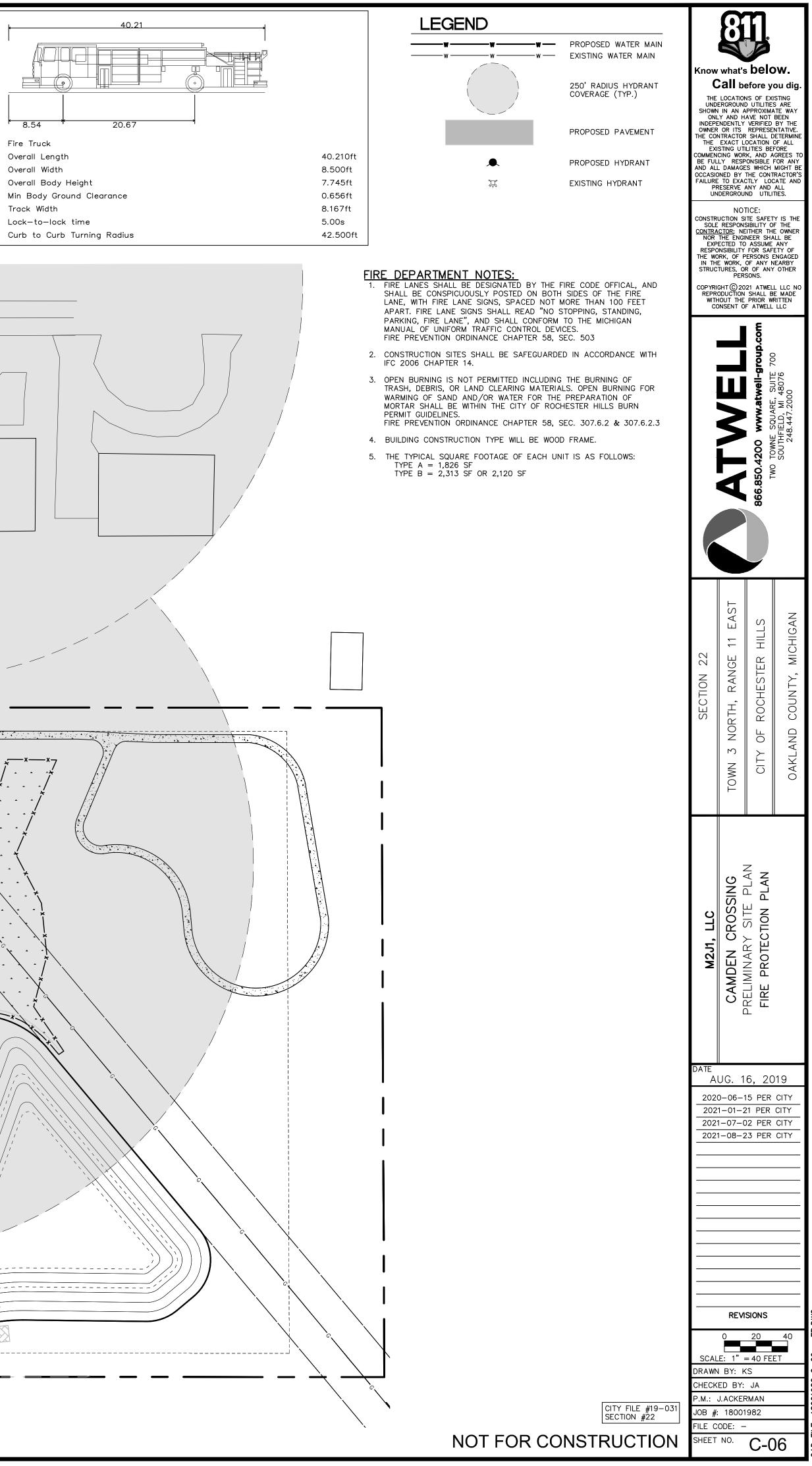
## Rochester Hills, Oakland County

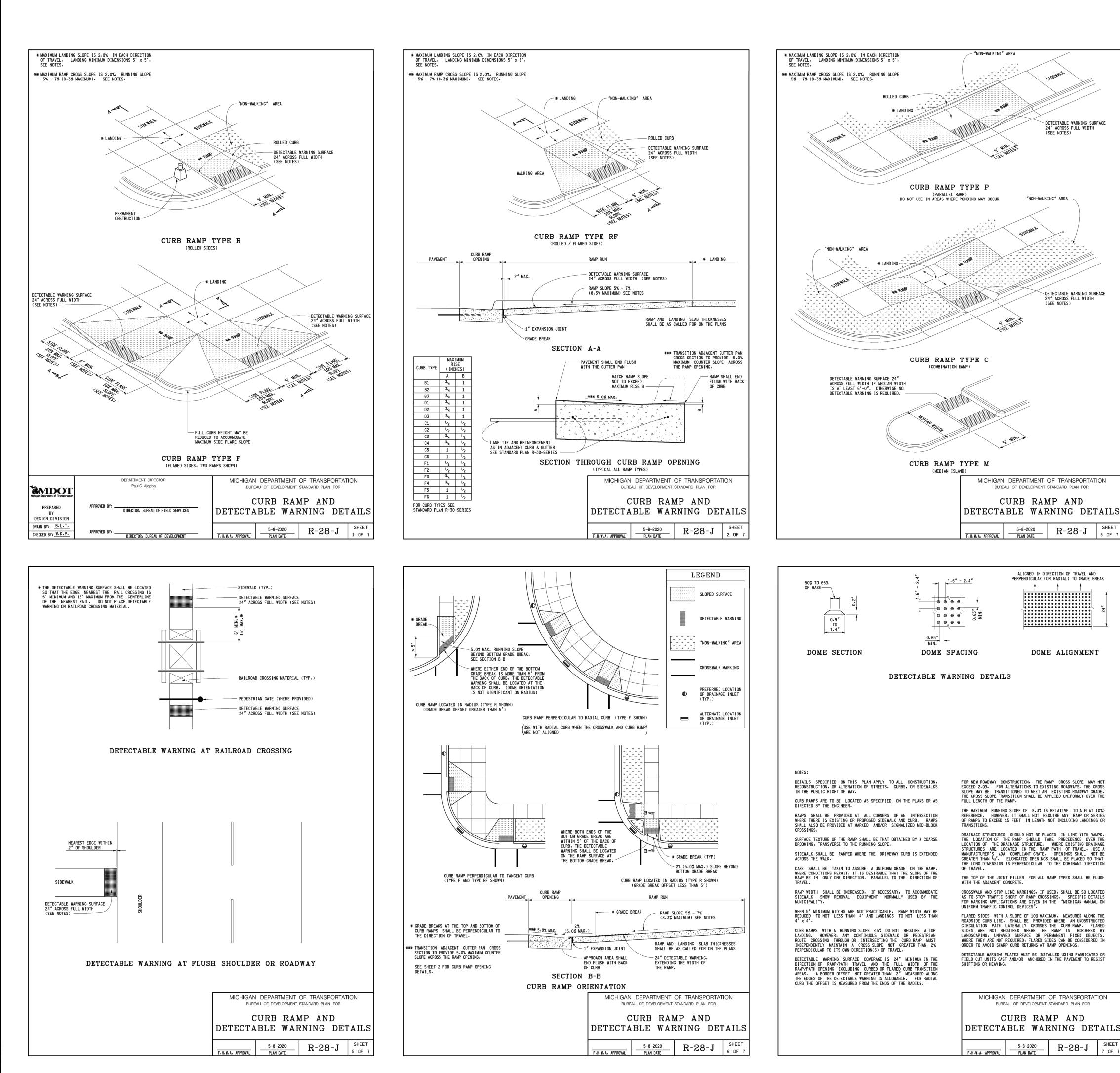
July 2, 2021

### Design Basis:

		Runoff				
Drainage Area Tributary to Basin	<u>Area (ac.)</u>	<u>Coefficient</u>				
Impervious Area (road, driveway, sidewalk, and roofs)	2.29	0.95				
Detention Pond	0.77	1.00				
Landscaped Areas (Hydrologic Soil Group B)	6.30	0.25				
Landscaped Areas (Hydrologic Soil Group B) Off Site Lots	1.60	0.25	Determination o	f Forebay Storage		
Impervious Off Site Lots	0.40	0.95	Determination	Trorebay Storage		
Total Area	11.36		Elevation	Area	Incr. Vol.	Cum. Vol.
Detention Area Sizing			(feet)	(sf)	(cft)	(cft)
Detention Area Sizing			( )	( )	( )	( )
1) Total Tributary Area	=	11.36	819.5	7,200		
Developed Runoff Coefficient (C)		0.47			3,925	3,925
Allowable Discharge Rate ( $C_0$ )	=	0.20 cfs/ac	820	8,500		
Anowable Discharge Nate (C. 6)		0.20 Charle			9,900	13,825
2) Maximum Allowable Runoff, Qa. = $C_0 \times A$	=	2.27 cfs	821	11,300		
		0.70 cfs			12,850	26,675
Maximum Allowable Runoff, Church Parcel Detention Calculations Total Maximum Allowable Runoff	=	2.97 cfs	822	14,400		
Total Maximum Allowable Runoff	—	2.97 cls			Total Volume=	26,675 c
3) Calculate Qo Qa / (a * c)		0.43 cfs/acre impervious.			<b></b>	
(max, outflow per acre impervious)			<b>Determination</b> o	f Storage Provided in Basin	Including Forebay Stora	ige)
			Elevation	Area	Incr. Vol.	Cum Vol
4) Max. Storage Time, $T = -25 \pm sqrt(8062.5/Qo)$	=	112.10 minutes	(feet)	(sf)	(cft)	(cft)
			(leet)	(31)	(cit)	((11)
5) Max. Storage Required, $Vs = ((12,900*T) / (T+25)) - 40QoT$	=	8,624 cf/acre impervious.	819.5	18,600		
			01710	10,000	10,000	10,000
6) Total Storage Required, Vt = Vs * a * c	=	45,685 cf	820	21,400	2	,
As-Built Volume of Existing Church Basin	=	10,541 cf		,	24,350	34,350
(Per As-Built St. Mark Church plans dated 8-19-1999)			821	27,300		
Total Storage Required	=	56,226 cf			30,550	64,900
Total Storage Required	_	30,220 (1	822	33,800		
The Live Control Live Control to					Total Volume=	64,900 c
7) 1-Year Storm Calculation for Forebay		22.004				
Total Storage ( $Vt = 4,320 \times A \times C$ )	=	22,884 cf	<u>Storage &amp; Eleva</u>	tion Summary		
				ear Storm Elevation		=
8) Sizing of Overflow Weir			Design 25-Year Storm Elevation			=
Q=CLH^3/2			Top of Bank Elevation			=
Q(CIA) = 10.62  cfs			Minimum Top of Bank Elevation			=
L = ? ft			Bankfull Volume: VBF=6,897 x C x A			=
H= 0.5 ft C= 3.367			Bankfull Elevation			
			Volume Provided			=
$L = Q/(3.367*H^{3/2}) =$	=	9 ft	Volume Requ			







MICHIGAN DEPARTMENT OF TRANSPORTATION BUREAU OF DEVELOPMENT STANDARD PLAN FOR CURB RAMP AND DETECTABLE WARNING DETAILS R-28-J SHEET 5-8-2020 PLAN DATE 7 OF F.H.W.A. APPROVAL

FOR NEW ROADWAY CONSTRUCTION, THE RAMP CROSS SLOPE MAY NOT EXCEED 2.0%. FOR ALTERATIONS TO EXISTING ROADWAYS, THE CROSS SLOPE MAY BE TRANSITIONED TO MEET AN EXISTING ROADWAY GRADE. THE CROSS SLOPE TRANSITION SHALL BE APPLIED UNIFORMLY OVER THE FULL LEWETH OF THE RAMP.

DETECTABLE WARNING SURFACE 24" ACROSS FULL WIDTH (SEE NOTES)

DETECTABLE WARNING SURFACE

24" ACROSS FULL WIDTH (SEE NOTES)

"NON-WALKING" AREA —

BUREAU OF DEVELOPMENT STANDARD PLAN FOR

CURB RAMP AND

R-28-J

ALIGNED IN DIRECTION OF TRAVEL AND PERPENDICULAR (OR RADIAL) TO GRADE BREAK

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DOME ALIGNMENT

5-8-2020

PLAN DATE

SHEET

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FULL LENGTH OF THE RAMP.

THE MAXIMUM RUNNING SLOPE OF 8.3% IS RELATIVE TO A FLAT (0%) REFERENCE. HOWEVER, IT SHALL NOT REQUIRE ANY RAMP OR SERIES OF RAMPS TO EXCEED 15 FEET IN LENGTH NOT INCLUDING LANDINGS OR

TRANSITIONS.

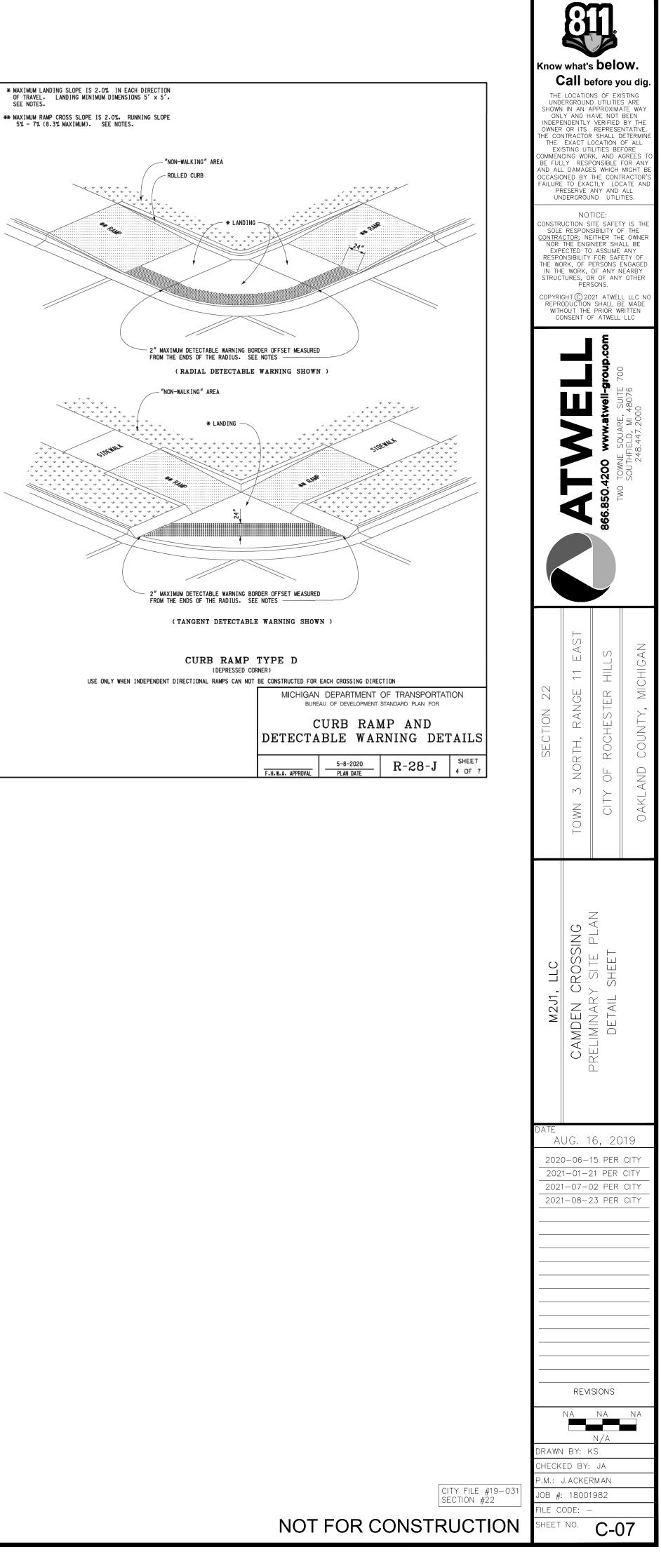
DRAINAGE STRUCTURES SHOULD NOT BE PLACED IN LINE WITH RAMPS. THE LOCATION OF THE RAMP SHOULD TAKE PRECEDENCE OVER THE LOCATION OF THE DRAINAGE STRUCTURE. WHERE EXISTING DRAINAGE STRUCTURES ARE LOCATED IN THE RAMP PATH OF TRAVEL, USE A MANUFACTURER'S ADA COMPLIANT GRATE. OPENINGS SHALL NOT BE GREATER THAN  $\frac{1}{2}$ . ELONGATED OPENINGS SHALL BE PLACED SO THAT THE LONG DIMENSION IS PERPENDICULAR TO THE DOMINANT DIRECTION OF TRAVEL.

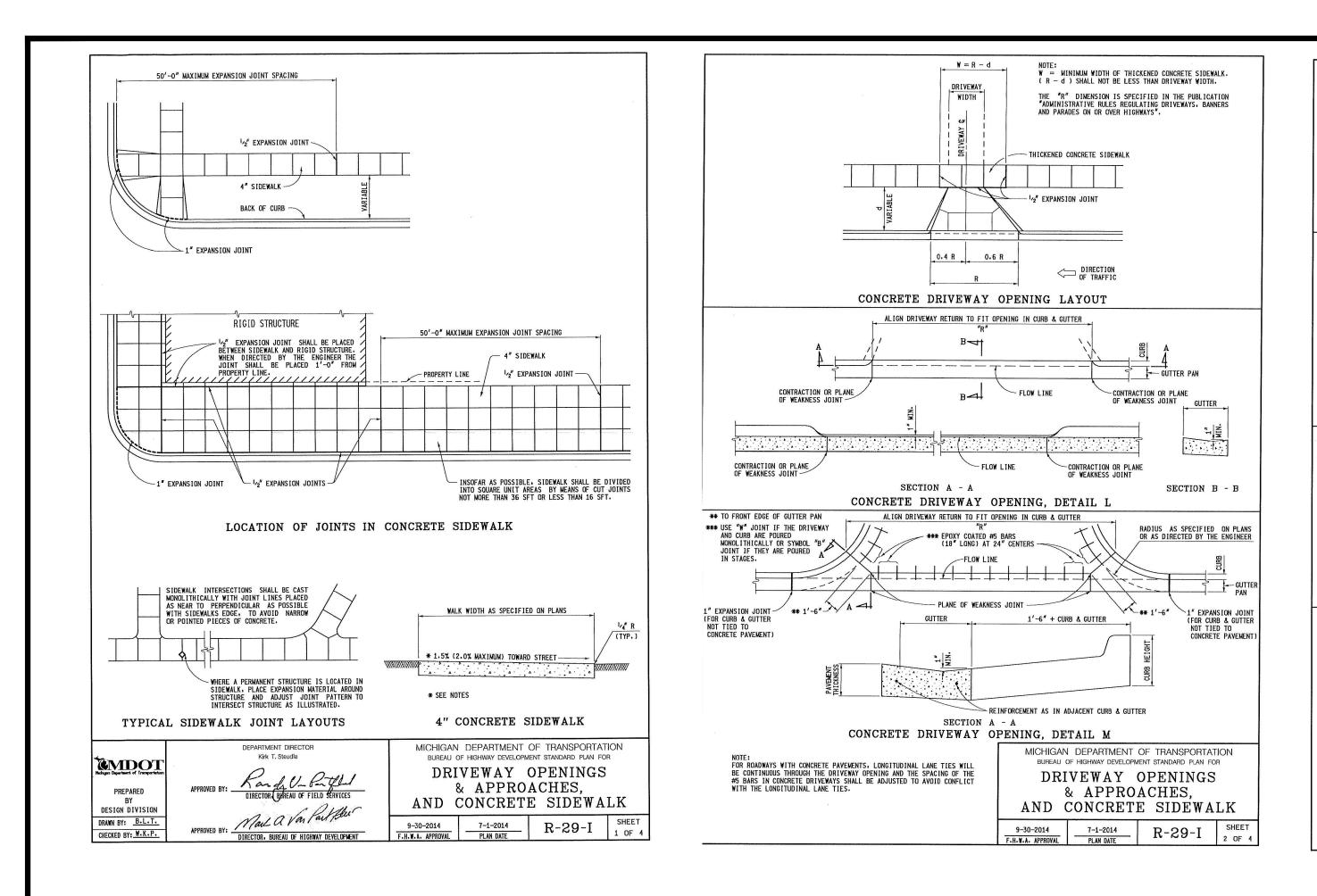
THE TOP OF THE JOINT FILLER FOR ALL RAMP TYPES SHALL BE FLUSH WITH THE ADJACENT CONCRETE.

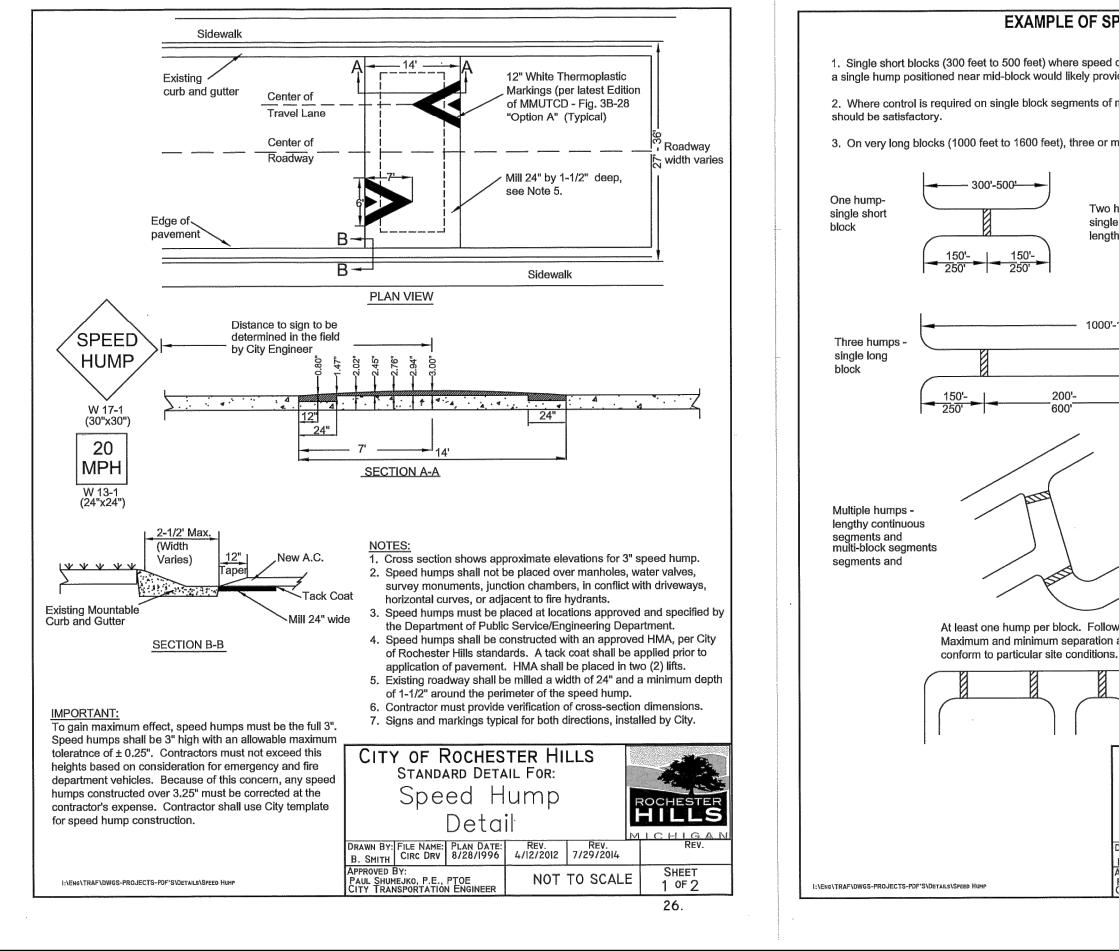
CROSSWALK AND STOP LINE MARKINGS, IF USED, SHALL BE SO LOCATED AS TO STOP TRAFFIC SHORT OF RAMP CROSSINGS. SPECIFIC DETAILS FOR MARKING APPLICATIONS ARE GIVEN IN THE "MICHIGAN MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES".

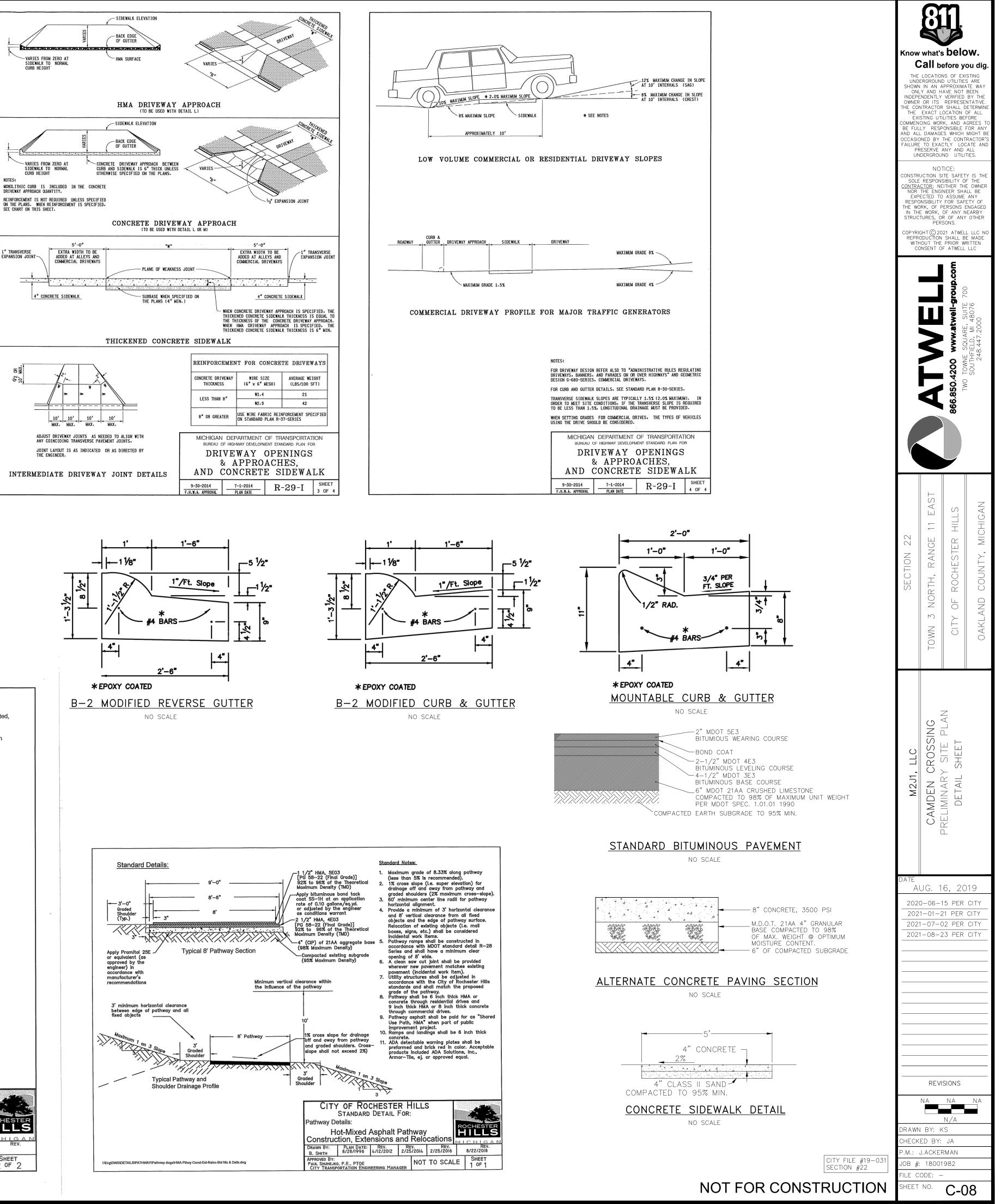
FLARED SIDES WITH A SLOPE OF 10% MAXIMUM, MEASURED ALONG THE ROADSIDE CURB LINE, SHALL BE PROVIDED WHERE AN UNOBSTRUCTED CIRCULATION PATH LATERALLY CROSSES THE CURB RAMP. FLARED SIDES ARE NOT REQUIRED WHERE THE RAMP IS BORDERED BY LANDSCAPING, UNPAYED SURFACE OR PERMANENT FIXED OBJECTS. WHERE THEY ARE NOT REQUIRED, FLARED SIDES CAN BE CONSIDERED IN OPDER TO AVOID SHAPP CUBB RETURDS AT RAMP OPENINGS.

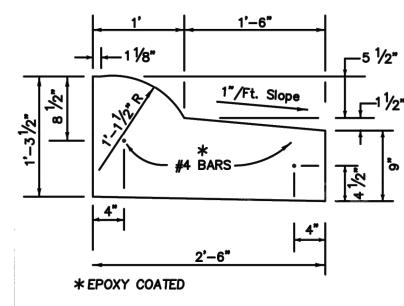
ORDER TO AVOID SHARP CURB RETURNS AT RAMP OPENINGS. DETECTABLE WARNING PLATES MUST BE INSTALLED USING FABRICATED OR FIELD CUT UNITS CAST AND/OR ANCHORED IN THE PAVEMENT TO RESIST

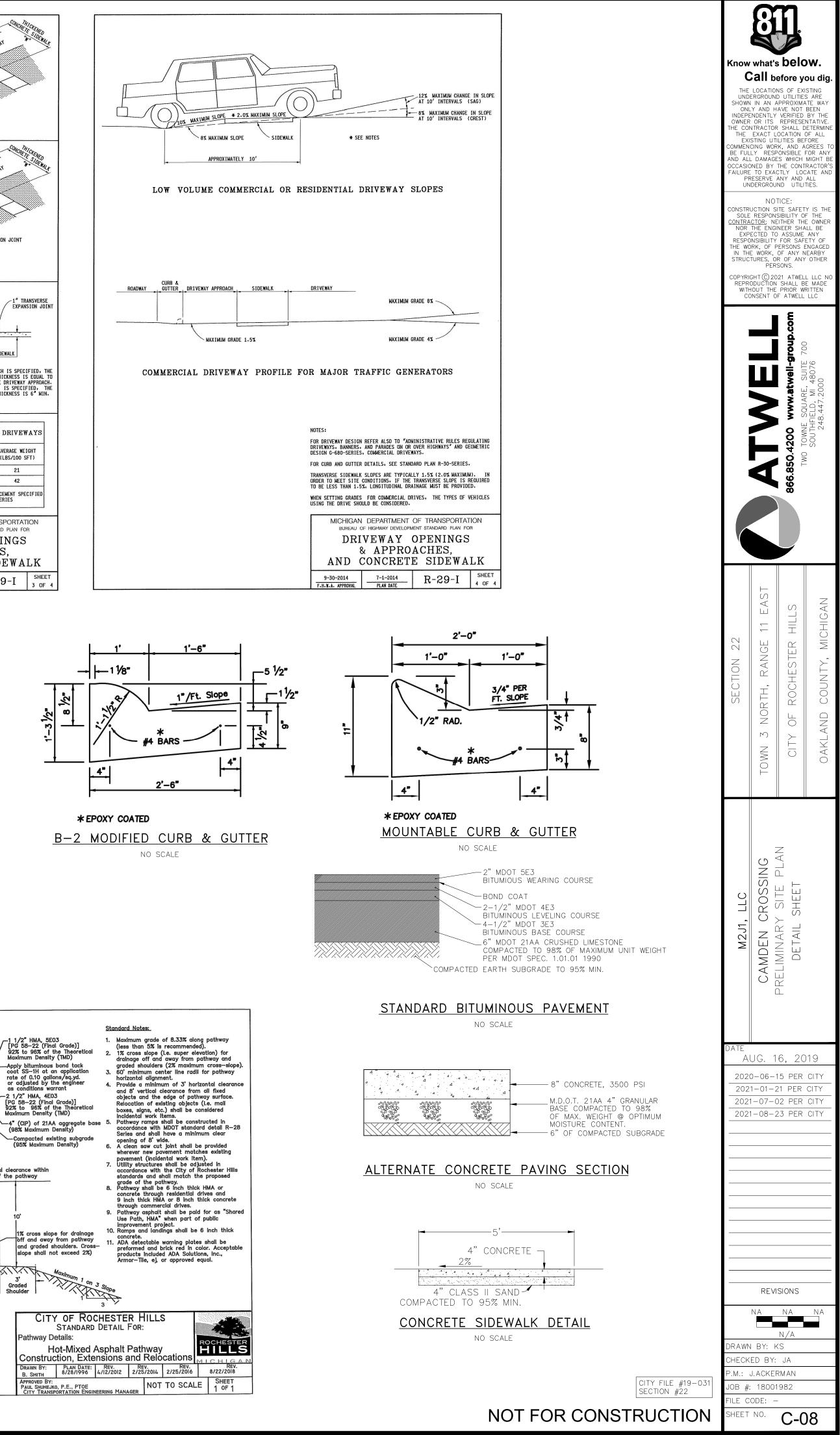


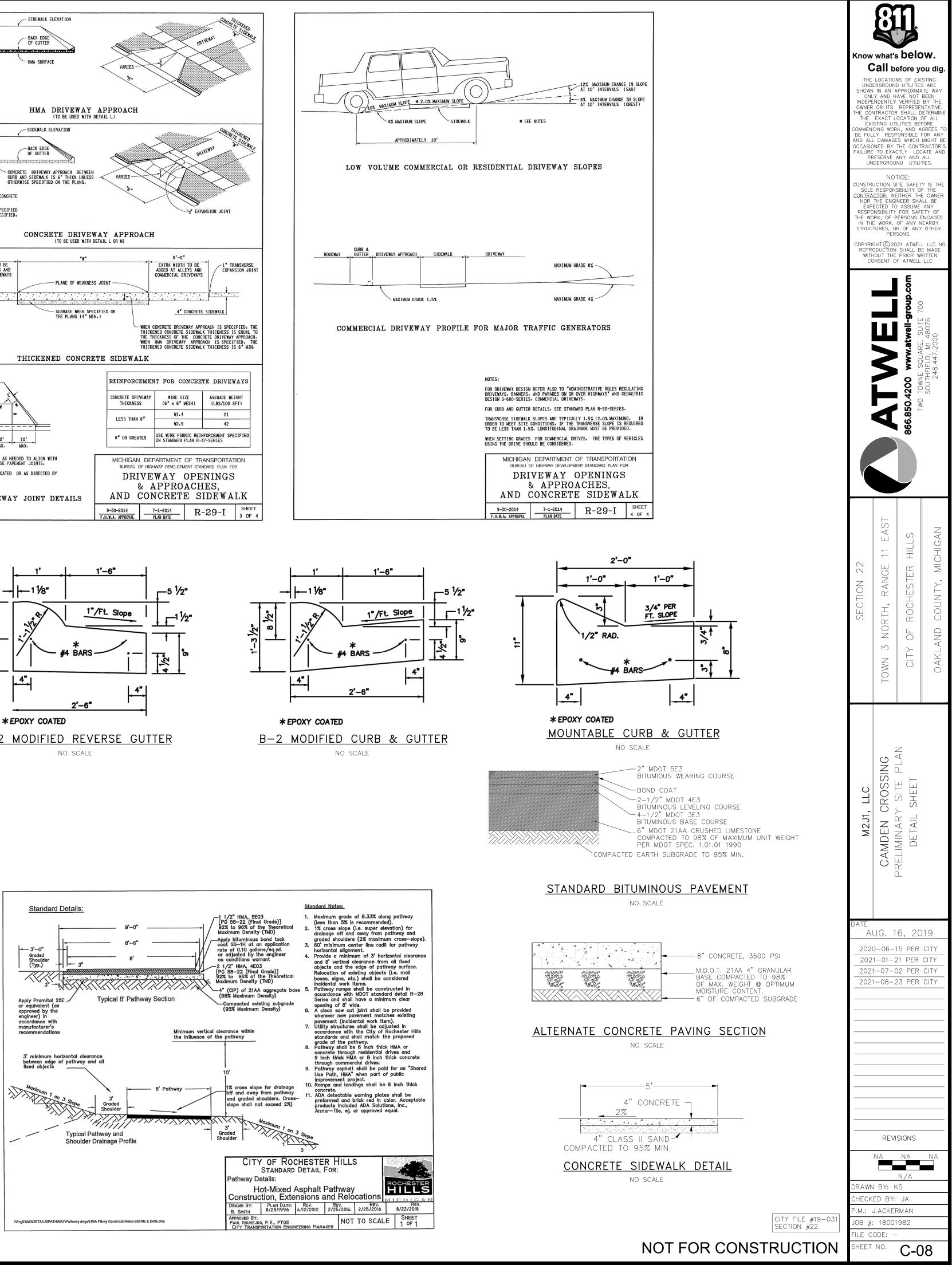












## EXAMPLE OF SPEED HUMP LOCATIONS

1. Single short blocks (300 feet to 500 feet) where speed control problems area unusual. Where such blocks must be treated, a single hump positioned near mid-block would likely provide satisfactory speed control over the intire block. 2. Where control is required on single block segments of moderate length (500 feet to 1000 feet), a two-hump configuration

3. On very long blocks (1000 feet to 1600 feet), three or more humps might be necessary. 300'-500' -----Two humps single moderate length blosk 200'-600 - 1000'-1600' At least one hump per block. Follow spacing concepts above within each component block. Maximum and minimum separation and "first hump" criteria may be relaxed somewhat to CITY OF ROCHESTER HILLS STANDARD DETAIL FOR: Speed Hump Location Detail DRAWN BY: FILE NAME: PLAN DATE: REV. REV. B SMITH SPEED HMP 8/28/1996 4/12/2012 3/15/2014 B. SMITH NOT TO SCALE PAUL SHUMEJKO, P.E., PTOE CITY TRANSPORTATION ENGIN